

## ENTERGY INDEPENDENCE PLANT LANDFILL CELLS 12 – 15

## DEMONSTRATION OF COMPLIANCE WITH EPA CCR RULE SITING CRITERIA §257.64, UNSTABLE AREAS

PREPARED IN COMPLIANCE WITH THE EPA FINAL RULE FOR THE DISPOSAL OF COAL COMBUSTION RESIDUALS TITLE 40 CODE OF FEDERAL REGULATIONS PART 257



**OCTOBER 17, 2018** 

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#### DEMONSTRATION OF COMPLIANCE WITH EPA CCR RULE SITING CRITERIA §257.64, UNSTABLE AREAS

Prepared for

Entergy Arkansas, Inc. PO Box 551 Little Rock, AR 72203

Prepared by

FTN Associates, Ltd. 3 Innwood Circle, Suite 220 Little Rock, AR 72211

FTN No. R07920-1872-001

October 17, 2018

#### **PROFESSIONAL ENGINEER'S CERTIFICATION**

With this certification, I certify that I, as a Professional Engineer in the State of Arkansas, am a qualified professional engineer as defined in §257.53 of Title 40 Code of Federal Regulations (40 CFR) Part 257, that this report has been prepared under my direction in accordance with generally accepted good engineering practices, that the findings are accurate to the best of my knowledge, and that the CCR unit that is subject to this certification meets the location restriction requirements under §257.64 of 40 CFR Part 257.



Dana L. Derrington, Arkansas PE #16372

10/17/2018 Date

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#### **1.0 INTRODUCTION**

Entergy Arkansas, Inc. (Entergy), operates the Independence plant located approximately 2 miles southeast of Newark, Arkansas. The plant utilizes four disposal cells, Cells 12 through 15, hereafter also referred to as the landfill, for the disposal of coal combustion residuals (CCRs) generated from the combustion of coal at the plant. Pursuant to §257.64 of Title 40 Code of Federal Regulations (40 CFR) Part 257, existing CCR landfills must not be located in an unstable area. An unstable area is defined by §257.53 as a location that is susceptible to natural or human-induced events or forces capable of impairing the integrity, including structural components of some or all of the CCR unit that are responsible for preventing releases from such unit. Unstable areas can include poor foundation conditions, areas susceptible to mass movements, and karst terrains. This report presents the findings of an evaluation of Cells 12 through 15 in support of the location restriction requirements of §257.64.

#### 2.0 SITE DESCRIPTION

Per the CCR rule, an existing CCR unit is defined as a unit that "receives CCR both before and after October 19, 2015 or for which construction commenced prior to October 14, 2015." The CCR unit received CCR before and after October 19, 2015, and no lateral expansions have occurred after October 19, 2015. Thus, the Cells 12 through 15 are an existing landfill per the CCR rule.

The combined area of Cells 12 through 15 is approximately 45 acres with a maximum elevation of 284 ft North American Vertical Datum of 1988 (NAVD88) as of the date of the last survey, which was completed in November 2017. Natural topography surrounding the landfill is generally flat-lying, with ground surface elevations ranging from approximately 235 to 237 ft NAVD88, as shown on Figures 1 and 2 (Appendix A).

### **3.0 UNSTABLE AREA EVALUATION**

Pursuant to §257.64(b), the owner or operator must consider all of the following factors, at a minimum, when determining whether an area is unstable:

- 1. Onsite or local soil conditions that may result in significant differential settling;
- 2. Onsite or local geologic or geomorphologic features; and
- 3. Onsite or local human-made features or events (both surface and subsurface).

FTN Associates, Ltd. (FTN) performed a review of site-specific boring logs, geotechnical data, US Geological Survey (USGS) publications, and documentation related to the landfill's Solid Waste Permit No. 0200-S3N-R2 issued by the Arkansas Department of Environmental Quality. Findings from this review are discussed below within the context of the factors listed in §257.64(b).

#### 3.1 Review of Onsite or Local Soil Conditions

Several subsurface investigations have been performed in the vicinity of the landfill. Available soil boring logs and geotechnical data (Appendix B) show that onsite soils are comprised of low- to high-plasticity clays and low-plasticity silts to an approximate depth of 30 ft below ground surface (bgs) followed by sands and gravels that extend to an approximate depth of 130 ft bgs. These soils are bounded below by Paleozoic rocks and associated residuum (Albin, Hines, and Stephens 1967). A review of the subsurface data included in Appendix B showed that no organic soils, which are prone to settlement due to their high compressibility, were encountered in any of the borings. There were also no apparent lateral changes in the underlying lithology that would indicate a notable change in the compressibility of foundation soils, as can be seen from the soil boring logs. These factors, coupled with a review of the settlement calculations performed for Permit No. 0200-S3N-R2, indicate that significant differential settling is unlikely.

#### 3.2 Review of Onsite or Local Geologic or Geomorphologic Features

Surficial deposits in the vicinity of the landfill are generally comprised of Quaternary alluvial and terrace deposits as shown by the geological map included as Figure 3. A review of the area topography (Figures 1 and 2) and the geological map shows no evidence of karst features or areas susceptible to mass movement (i.e., landslides) in the vicinity of the landfill.

# 3.3 Review of Onsite or Local Human-Made Features or Events (Both Surface and Subsurface)

Presently, there are no known visible onsite or local human-made features or events that would cause the area in the immediate vicinity of the landfill to be unstable. The underlying sands and gravels described in Section 3.2 are part of the Mississippi River Valley alluvial aquifer, which is used extensively in the vicinity of the plant for agricultural purposes. However, a review of an ongoing study conducted by USGS (Schrader 2015) indicates that recharge to aquifer is sufficient in the vicinity of the plant to balance seasonal withdrawals. As such, land subsidence due to groundwater removal is considered unlikely.

#### **4.0 CONCLUSIONS**

Based on a review of the available documentation in this report, Cells 12 through 15 at the Entergy Independence plant are not located in an unstable area and therefore meet the location restriction requirements of §257.64.

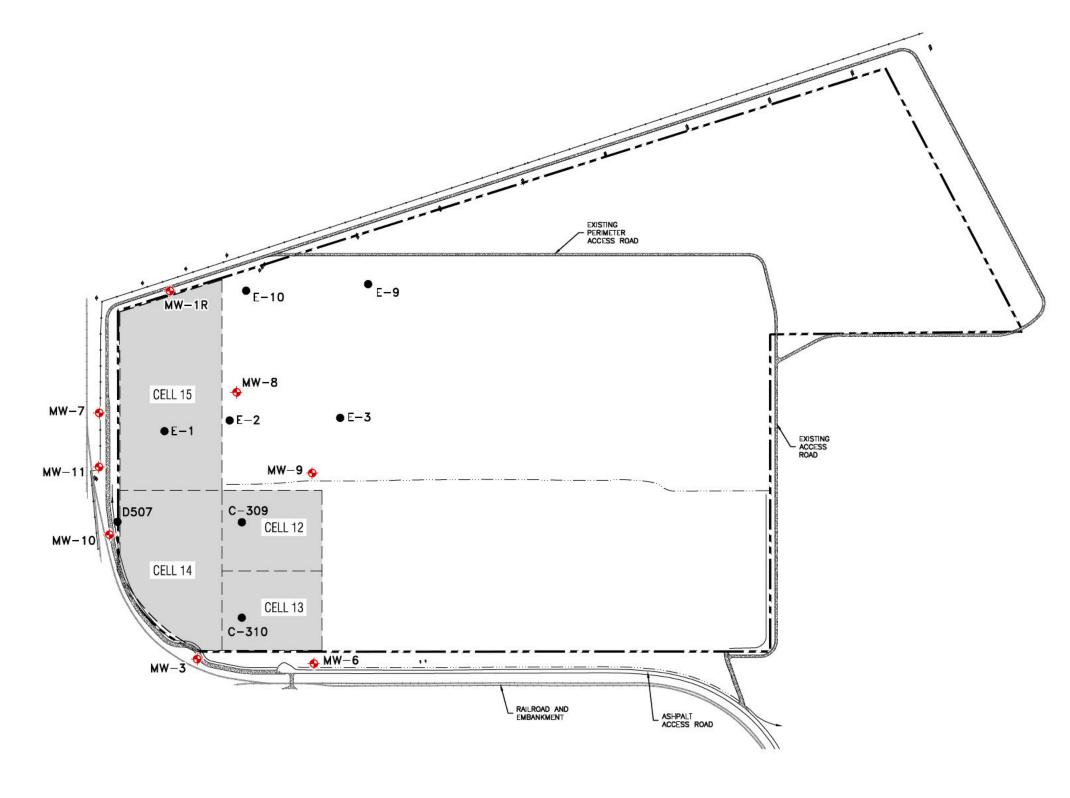
#### **5.0 REFERENCES**

Albin, D.R., M.S. Hines, and J.W. Stephens. 1967. Water Resources of Jackson and Independence Counties, Arkansas [USGS Water Supply Paper 1839-G]. Prepared in cooperation with the Arkansas Geological Commission. Washington, DC: United States Government Printing Office. 39 pp.

- Schrader, T.P. 2015. Water Levels and Water Quality in the Mississippi River Valley Alluvial Aquifer in Eastern Arkansas, 2012 [USGS Scientific Investigations Report 2015–5059]. Prepared in cooperation with the Arkansas Natural Resources Commission and the Arkansas Geological Survey. Reston, VA: US Geological Survey. <u>http://dx.doi.org/10.3133/sir20155059</u>.
- Stoeser, D.B., G.N. Green, L.C. Morath, W.D. Heran, A.B. Wilson, D.W. Moore, and B.S. Van Gosen. 2005. "The State of Arkansas." In *Preliminary Integrated Geologic Map Databases for the United States Central States: Montana, Wyoming, Colorado, New Mexico, Kansas, Oklahoma, Texas, Missouri, Arkansas, and Louisiana* [USGS Open-File Report 2005-1351]. Denver, CO: US Geological Survey. Available online at <u>http://pubs.usgs.gov/of/2005/1351/</u>.
- USGS [US Geological Survey]. 1962 (rev 1981). "USGS 1:24000-Scale Quadrangle for Newark, AR 1962." US Geological Survey. Available online at https://www.sciencebase.gov/catalog/item/5a8a29e6e4b00f54eb3c797b.

# **APPENDIX** A

Figures



R:\projects\07920-1872-001\cad\dwg\07920-1872-001-FG01.DWG

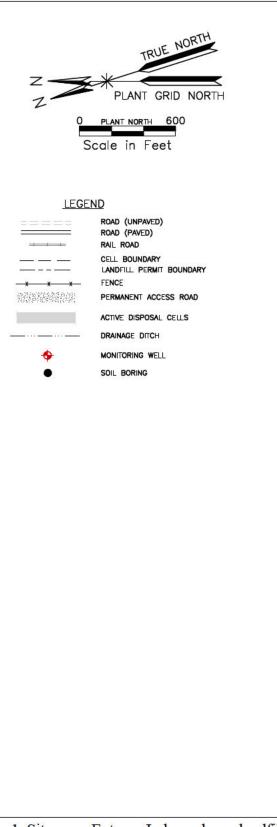


Figure 1. Site map, Entergy Independence landfill.

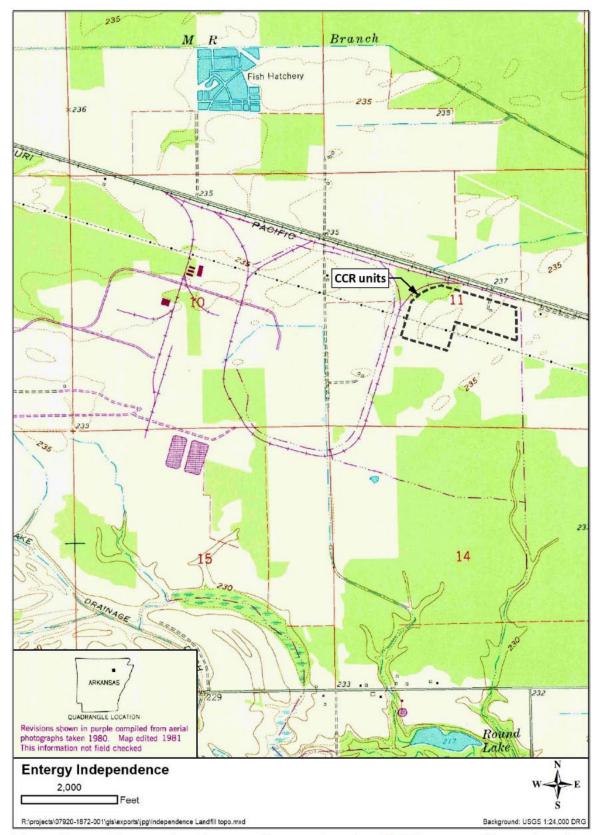


Figure 2. Topography of surrounding area based on USGS topographic quadrangle Newark, AR (1981).

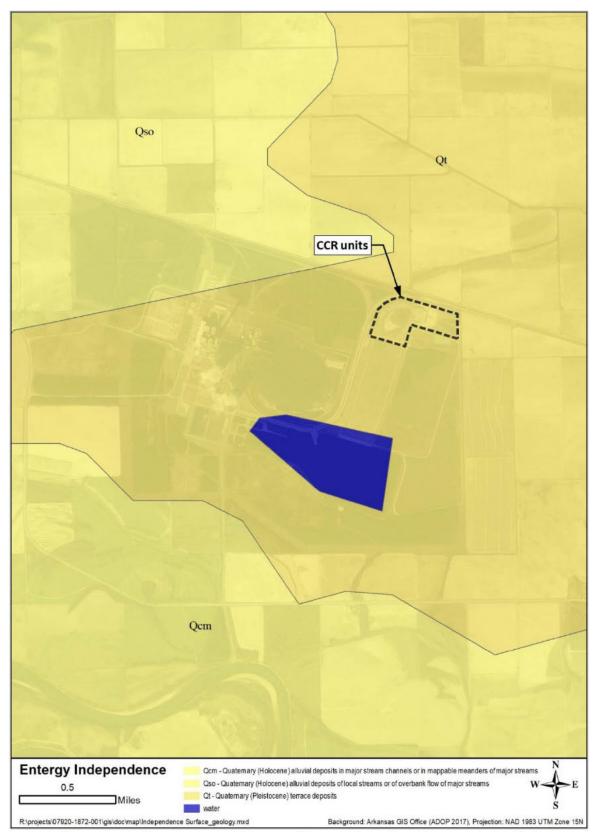
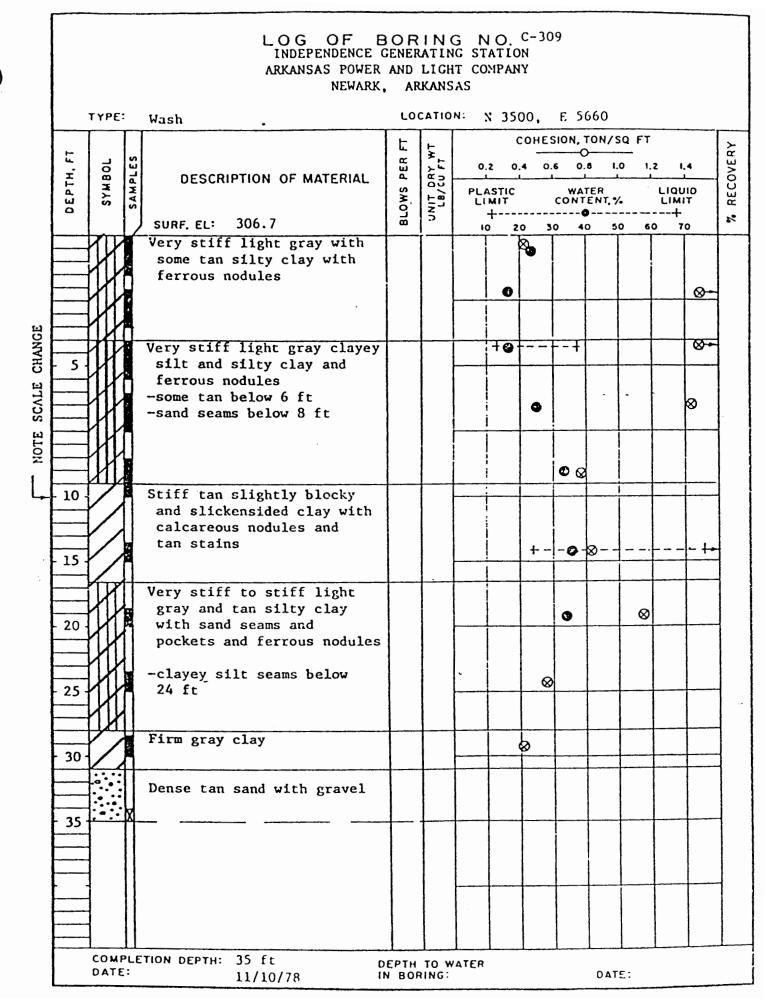


Figure 3. Surface geology of surrounding area based on Stoeser et al. 2005.

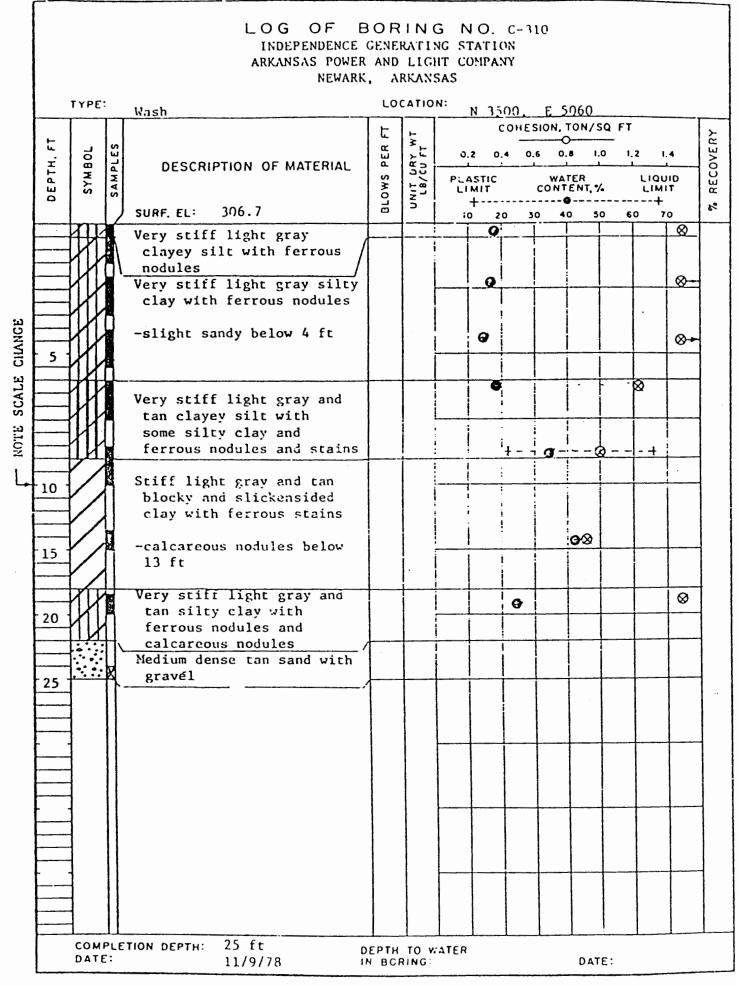
## **APPENDIX B**

Well Construction Diagrams, Soil Boring Logs, and Geotechnical Data

Well Construction Diagrams and Soil Boring Logs



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Boring #	¥: E−1	<u> </u>	Location:	ENTERGY -	Independe	ence Plant		
Date: 4/	/2/01	FTN Associates, Ltd.	Drilling Me	thod: HSA				
Elevation	n: 308	Little Rock, Arkansas	Driller: Tri	State Testing	g, Inc.			
Job No.:	6040-320	de manufacturation de 3 innwood Circle, Suite 220, 72211	Logged By: MSR					
Depth, Feet	Litho. Symbol	Classification	Sample Type	Blow Counts	PPR	Depth, Feet		
		0.0 – 6.0 Stiff, tan, dry to moist silty clay with ferrous nodules	SHELBY TUBE	N/A	N/A			
5			SPLIT SPOON	10,12,14	4.5	5		
		6.0 – 8.5 Medium stiff, tan, dry silt	SPLIT SPOON	6,8,10	1.5			
<i>10</i>		8.5 – 24.0 Soft, brown, moist clay with ferrous staining	SPLIT SPOON	2,2,3	0.5			
			SHELBY TUBE	N/A	N/A			
15			SPLIT SPOON	3,5,6	1.5	15		
20		@ 18.5 — 20.0 calcareous nodules	SPLIT SPOON	<b>4,3</b> ,5	1.75	20		
25		24.0 - 34.0 Loose, light brown $\sum_{=}$ and gray, wet, fine sand with some $\equiv$ silt and clay	SPLIT SPOON	2,2,3	0.5	25		
30			SPLIT SPOON	2,1,5	N/A	<i>30</i>		
35	<u>e O.S</u> ev	34.0 – 35.0 Medium dense, light brown wet, fine to coarse sandy gravel Boring terminated @ 35 ft bgs	SPLIT SPOON	16,16,12	N/A	35		
40						40		
- - 						45		

Boring #	<b>#</b> : E−2		Location:	Entergy -	Independe	nce Plant
Date: 04		FTN Associates, Ltd.		thod: HSA		
Elevation		Little Rock, Arkansas		State Test	ting. Inc.	
L	6040-320	wir mengebundt auf 3 innwood Circle, Suite 220, 72211	Logged By			
Depth, Feet	Litho. Symbol	Classification	Sample Type	Blow Counts	PPR	Depth, Feet
		0.0 — 23.0 Soft to stiff, tan to light gray, moist silty clay with ferrous	SPLIT SPOON	1,3,3	2.5	
		nodules and occasional very fine sand	SHELBY TUBE	N/A	N/A	
5 				455	2.25	5 
			SPLIT SPOON	4,5,6	2.25	
10			SPLIT SPOON	3,5,5	2.0	10
_			SHELBY TUBE	N/A	N/A	— —
15			SPLIT SPOON	5, 5, 7	2.5	15
20			SPLIT SPOON	4,5,10	1.5	20
25		¥ 23.0 − 24.5 Medium dense, tan, <sup>=</sup> wet, clayey fine sand	SPLIT SPOON	11,15,10	N/A	25
	0.00.00	24.5 – 30.0 Medium dense, light brown, wet, fine to coarse sandy gravel with some clay				
			SPLIT SPOON	9,7,11	N/A	30
		Boring terminated @ 30 ft bgs				
35						35 
40						40
- -						
45						45

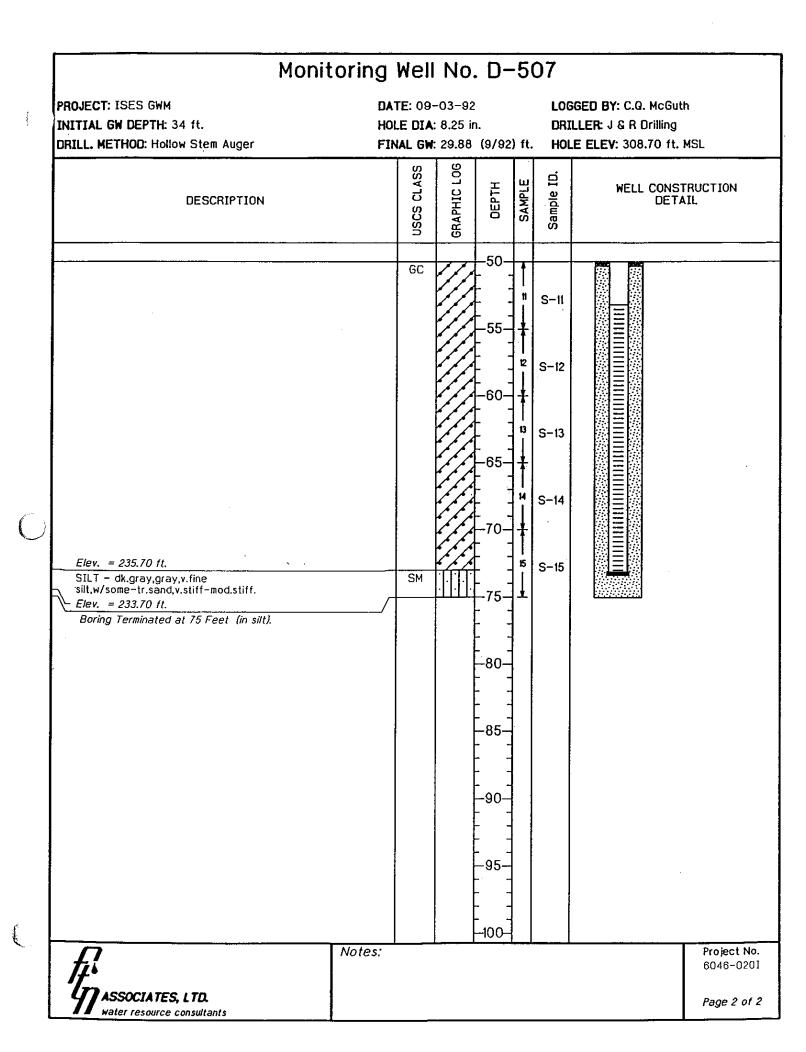
Boring #	₩ F-3		Location:	Entergy -	Independe	nce Plant
Date: 04		FTN Associates, Ltd.		thod: HSA		
Elevation		Little Rock, Arkansas		State Test	tina. Inc.	
	6040-320	3 Innwbod Circle, Suite 220, 72211	Logged By		,	
Depth, Feet	Litho. Symbol	Classification	Sample Type	Blow Counts	PPR	Depth, Feet
		0.0 – 29.5 Soft to stiff, gray to tan, dry to moist silty clay with ferrous	SPLIT SPOON	2,2,3	1.5	
		staining	SPLIT SPOON	5,8,13	3.5	
5						— 5
			SHELBY TUBE	N/A	N/A	
- 10			SPLIT SPOON	7,7,8	3.5	10
15		@ 18.5 –20.0 occasional calcareous	SHELBY TUBE	N/A	N/A	15
<u> </u>		nodules				 
└── 20			SPLIT SPOON	4,3,4	2.5	20
		Ž				
25		=	SPLIT SPOON	4, 4, 4	2.25	25
30	0.0.0	29.5 – 33.5 Medium dense, brown, wet,	SPLIT SPOON	6,21,18	N/A	30
		fine to coarse sandy gravel				
35	20002 2002	33.5 – 34.5 Loose, brown, wet, fine to coarse sand	SPLIT SPOON	8,8,7	N/A	
		34.5 – 35.0 Loose, brown, wet, fine sandy gravel				
40		Boring terminated @ 35 ft bgs				 40
-						
45						45

Boring #	#: F−9		Location:	Entergy –	Independe	nce Plant		
Date: 04		FTN Associates, Ltd.		thod: HSA				
Elevation		Little Rock, Arkansas			ting. Inc.			
·	6040-320	ASSOCIES IA. 3 Innwood Circle, Suite 220, 72211	Driller: Tri State Testing, Inc. Logged By: MSR					
Depth, Feet	Litho. Symbol	Classification	Sample Type	Blow Counts	PPR	Depth, Feet		
		0.0 — 23.5 Medium stiff, tan and gray, moist silty clay with ferrous staining	SPLIT SPOON	2,3,3	1.25			
5			SPLIT SPOON	4,6,7	1.0	5		
			SHELBY TUBE	N/A	N/A			
10			SPLIT SPOON	5,7,9	2.5	10		
			SPLIT SPOON	3,4,5	2.0	15		
			SHELBY TUBE	N/A	N/A			
20			SPLIT SPOON	4,6,5	1.5	20		
25		23.5 – 25.0 Very loose, brown, moist to wet clayey sand with ferrous	SPLIT SPOON	2,3,2	0.5	25		
	\$000 000 000	staining 25.0 — 30.0 Medium dense, brown, wet, fine to coarse sandy gravel with some						
30		clay Boring terminated @ 30 ft bgs	SPLIT SPOON	8,7,10	N/A	30		
		boring terminatea @ 50 nt bgs						
35						35		
F						 		
40						40		
- 45						45		

Boring #	<b>#</b> : F−10		Location:	Entergy –	Independe	nce Plant		
Date: 04		FTN Associates, Ltd.		thod: HSA	macpenae			
Elevation		Little Rock, Arkansas			ting Inc			
	6040-320	ASSociety III, and a Innwood Circle, Suite 220, 72211	Driller: Tri State Testing, Inc. Logged By: MSR					
Depth, Feet	Litho. Symbol	Classification	Sample Type	Blow Counts	PPR	Depth, Feet		
		0.0 – 23.5 Soft to stiff, tan and gray,				_		
		moist silty clay with ferrous staining	SPLIT SPOON	2,4,5	2.0			
5			SPLIT SPOON	6,8,9	2.0	5		
			SPLIT SPOON	3, 4, 4	1.5			
						<u> </u>		
10			SHELBY TUBE	N/A	N/A	10		
- 15			SPLIT SPOON	4,4,7	2.25			
			SHELBY TUBE	N/A	N/A			
20						20		
		27.5 25.0 /						
25		23.5 – 25.0 Loose, brown, moist to wet sandy clay with ferrous staining	SPLIT SPOON	1,2,6	N/A	25		
	2000000	25.0 – 27.5 Loose, brown, wet, fine sand 27.5 – 30.0 Medium dense, brown, wet	-					
30		fine to coarse sandy gravel with some clay	SPLIT SPOON	8,11,18	N/A			
		Boring terminated @ 30 ft bgs						
35						35 		
						_		
40						40		
						45		

Monitorir	_							
PROJECT: ISES GWM INITIAL GW DEPTH: 34 ft.			-03–92 : 8.25 in				: C.Q. McC & R Drillin	
DRILL. METHOD: Hollow Stem Auger	FIN	AL GW	29.88	(9/92)	) ft.		308.70 1	
DESCRIPTION		USCS CLASS	GRAPHIC LOG	DEPTH	SAMPLE	Sample ID.	WEL	L CONSTRUCTIO
SILT - It.tan-brown (topsoil),organics present (grass,roothairs),v.stiff /dense,sl.moist. <i>Elev. = 307.70 ft.</i> CLAY - It.tan,tan-buff,v.stiff,mottled,granular texture in zones,sl-tr silt,blocky texture in some areas, no organics present,sl.moist.		ML CH		- 0	2	S-1 S-2 S-3		
					***** 4 · · *** 5 · · ***	S-4 S-5		· ·
Elev. = 274.70 fl.	¥				6	S-6 S-7		
Gravel – It.tan.brown,some silt,some sand,some chert,saturated. – grains ranging in size from granule – pebble (not exceeding 3'').		GC		-35-     	8	S-8		
					9	S-9 S-10		
F. Note	es.			-50-	<u>+</u>			Project No 6046-020
ASSOCIATES, LTD.								Page 1 of 2

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±		PROJEC Mon LOCATI	itoring Well Installation	WELL			
			DRILLING CONTRACTOR:		/-1R THING: 64.3 UND SURF/	ACE ELEV	EASTING: 3899.0 /.: TOC ELEVATION:
ASS water resources / environ	OCICITES imental consul	DRILLIN	RETHOD:	TOTA	.2 ft SR		<b>313.28 ft SRE</b> DEPTH TO WATER: (1/27/2016
LOGGED BY:		SAMPL	8.25" Hollow Stem Auger (HSA) SAMPLING METHOD: 5-ft continuous split barrel sampler		2 ft fron STARTED 9/2016		20.34 ft below TOC DATE COMPLETED: 1/20/2016
Depth (feet)	nscs	Graphic Log	Description	1/1/1	0/2010		Well
0 - 33			Non-CCR roadbase.				Above ground completion including 2x2 ft concrete pao four pipe bollards, and lockir outer steel casing
4 - 4 - 4 -	FILL		LEAN CLAY, silty, light grey with carbanceous material and iron oxide medium stiff, moist.	staining,			<ul> <li>35.8 ft of 2 in dia., Sch. 40</li> <li>PVC solid riser, including 3.1</li> <li>of above ground stickup</li> </ul>
8 -	CL		@8 ft bgs SAA, increasing silt with depth.				Cement/bentonite grout from
12 -	ML CH		CLAYEY SILT with fine-grained sand, grey with orange mottling and carbanaceous material, moist. FAT CLAY with silt, greyish brown with iron oxide staining and carbanc material, stiff, moist. LEAN CLAY, with silt, light grey with iron oxide staining and carbanceo				0 ft bgs to 23 ft bgs
16 - <sup>100</sup>	CL		material, stiff, moist. @17 ft bgs granular calcite concretions. FAT CLAY to LEAN CLAY with silt, light grey with iron oxide staining, s	tiff,			-
20 - <sub>100</sub>	CL/CH		moist.				-
24 – _ 90	CL		SANDY CLAY, fine-grained, brownish grey, increasing sand content wi soft, very moist. POORLY GRADED SAND, fine- to medium-grained, light brown, loose saturated.				Bentonite pellet seal from 23 ft bgs to 26 ft bgs Silica size 10/20 filter pack
28 - 40	SP						from 26 ft bgs to 43 ft bgs
32 -	SC		@33-33.7ft bgs, SAA with gravel, coarse-grained, subangular. CLAYEY SAND, fine- to medium-grained, grey, dense, very moist.				10 ft of 2 in dia., 0.010 in sk Sch. 40 PVC screen
36 – <sup>78</sup>	GW SP	0000	WELL GRADED GRAVEL, fine- to coarse-grained, subangular, with fir medium-grained sand, loose, saturated. POORLY GRADED SAND, medium- to coarse-grained, angular to sub loose, saturated. WELL GRADED GRAVEL, fine- to coarse-grained (<40mm), angular to	angular,			-
40 – <sub>45</sub> -	GW	000 000	WELL GRADED GRAVEL, time- to coarse-grained (<40mm), angular to subrounded with angular sand, saturated.			8	0.35 ft, 2 in dia., Sch. 40 PV end cap 43 ft BOH

			PROJECT:	BORING	D:		
			Entergy Independence Plant 3N Landfill	MW-3			
			LOCATION:	WELL ID			
-			Newark, Arkansas	MW-3	3		
	<b>=</b> 5		DRILLING CONTRACTOR:			ane NAD83N)	EAST: (AR State Plane NAD83N)
			Tri-State Testing Services	2 N.Y. 18 1 Statistics	23.74		1492589.45
-	Ass	oclates Ltd.	DRILLING EQUIPMENT:		ND ELEV. (	SRE):	TOC ELEVATION (SRE):
water resou	urces / environ	mental consultants	CME 750x	310.4		/	313.54
			DRILLING METHOD:	632735996538	DEPTH of I	BORING:	DEPTH TO WATER from TOC
	roject # 0-0093-0	001	Hollow Stem Auger	44 ft			34.40 ft (7/24/2013)
		JUT	SAMPLING METHOD:	1222 (2012) A.S.	TARTED.		DATE COMPLETED:
	D BY:		5-foot continous sampler	7/16/2	and a state of the state of the		7/16/2013
82.50	100000					11-2 × 2207	
Depth (feet)	nscs	Graphic Log	Description	REC		W	ell
epth	N	Gra	Description	% F		Const	ruction
							auch for and in an early and four model in the
-4 –						-	
9 <u>44</u>							3.1 ft. of stick-up and
•						_	protective steel casing
0 —			SILT with clay and angular white chert gravel, gray, medium	1			PVC vented below cap
-	ML		stiff, dry.	100		- 100	35.3 ft of 2 in dia., Sch. 40
4	SP	7//////	SAND, brown to gray, loose, medium grained, dry. CLAY, gray, lean, stiff, dry to moist. @ 4-5 ft. moist.	-			PVC solid riser
4 –		//////	OLAT, gray, ican, sun, ary to most. W +o it moist.			XX [	Cement/Bentonite grout from
8	CL	//////		100		XX -	0 ft to 26 ft bgs
•				100		XX -	
8 —		1111111	SILT with clay and sand, gray with 15% orange mottling and	1		888 E.	
8 <u>40</u>	ML		ferrous staining, stiff, fine grained sand, dry. @ 9 ft. no sand, brown with 30% orange and black mottling,			- 1888	
10			soft to medium stiff, moist.	100		XX -	
12 –	1		FAT CLAY, gray with 10% ferrous stains, fat, stiff to hard, blocky texture, dry to moist.	No de Calendaria		XX L	
	-		Netzmann 👻 a degeneral senten 🦇 Senten 🦉 - Senten de general de la sen			- 188	
10	CH					XX -	
16 –	1	//////	@ 18.5 ft. gray with 30% orange mottling and ferrous staining.	100			
8	-		e 10.0 h. gruy war oo v orange motaning and remous stamming.			XX -	
20							
20 –		//////	CLAY with silt, gray with 30 - 50% orange mottling, medium stiff, fat to lean, dry to moist. @ 20 ft. angular gray			XX E .	
67	CL		concretions.	100		XX -	
24 —		//////					
24			CLAY with sand, orange to brown, medium stiff, fat to lean, fine to medium grained sand, dry.				
8 <del>7</del>	CL	//////	ine to medium granicu sanu, dry.	50		×>> -	
28 -	00	4444A	SAND with clay, yellow-orange, loose to medium dense, dry	-			
20	SC SP		to moist.				Bentonite pellet seal from 26 ft
1	SP	DR42812B	SAND, brown, medium to fine grained, loose, saturated. GRAVEL with sand and silt, brown to orange, rounded to	-		-	bgs to 28 ft bgs
32 -		80808	sub-angular, loose, medium grained sand, saturated.	5			Natural filter pack from 28 ft
2		02122120					bgs to 44 ft bgs
80	GM	22,52,52			E	-	10 ft of 2 in dia., 0.010 in slot,
36 -		808080					Sch. 40 PVC screen
		000000000000000000000000000000000000000		25	E		0.3 ft, 2 in dia., Sch. 40 PVC
		02102102					end cap
40 -		8818 	GRAVEL interlayered with sand and silt, saturated.	1			Drilling terminated at 44 ft bgs
	GM/SP			25			
33	0101/3P	000		20		-	
		9813 			583		
44 –		Loss sectores 30 20		1 I			

Į.			LOCAT	Monitoring Well Installation     MW-6       LOCATION:     WELL ID:       Entergy Independence Landfill     MW-6			
	ſ	tr		NG CONTRACTOR: ray Drilling, LLC	304	THING: 9.51 JND SURFACE ELEV.:	EASTING: 4777.76 TOC ELEVATION:
uator reciv	ASS	OCICIES Imental consult		750X	308	.5 ft SRE	310.89 ft SRE
maner record	and a contract	montal ectory	DRILLIN	NG METHOD: "Hollow Stem Auger		L DEPTH: 3 ft from TOC	DEPTH TO WATER: (8/20/2015)
0666	DBY:		3	ING METHOD:		STARTED:	28.54 ft below TOC DATE COMPLETED:
RSI		-	5-ft o	continous split barrel	8/19	9/2015	8/19/2015
Depth (feet)	% REC	nscs	Graphic Log	Description			/ell truction
0 -	n/a			Non-CCR road base, plugged augers to drill through to insitu s	oils.		Above ground completion including 2X2 ft concrete pad, four pipe bollards, and locking outer steel casing
4 -	100	СН		FAT CLAY with silt, light gray, medium stiff, moist. Increasing si S LT, light gray to brown with organic material and iron oxide sta stiff, moist.			34.9 ft of 2 in dia., Sch. 40 PVC solid riser, including 2.4 f
8 -	100	ML ML/CL		S LT to LEAN CLAY, gray, moist.			of above ground stickup Cement/bentonite grout from 0
12 - - 16 - - 20 -	ST n/a	СН		FAT CLAY to ELASTIC S LT, light gray, with trace fine- to media sand, moist (see note).	Jm-grained		ft bgs to 22.8 ft bgs
24 –	n/a	ML/CL		S LT to LEAN CLAY, light gray, moist. WELL GRADED SAND, fine- to coarse-grained.			Bentonite pellet seal from 22.8 ft bgs to 24.8 ft bgs
- 28 –		SW					Silica size 10/20 filter pack from 24.8 ft bgs to 43 ft bgs
32 – -	n/a	344					10 ft of 2 in dia 0.010 in olet
6 -	n/a		0000	WELL GRADED GRAVEL, fine- to coarse-grained, subrounded fine-to-medium-grained sand.	to angular,		10 ft of 2 in dia., 0.010 in slot, Sch. 40 PVC screen from 32.5 ft bgs to 42.5 ft bgs
4 <b>0</b> –	n/a	GW	0000 0000				0.35 ft, 2 in dia., Sch. 40 PVC end cap
44 –			10-2400 - 14 AU				43 ft BOH

	PROJE	ST:	BOP	ING ID:			
		itoring Well Installation	MW				
-	LOCATI		WEL				
<u> </u>	0.000.000.000		MW				
		rgy Independence Landfill		THING:	EASTING:		
		ray Drilling, LLC		0.24			
		IG EQUIPMENT:	0010000000	UND SURFACE ELEV.:	6342.87 TOC ELEVATION:		
Associates	LL-L	<b>750X</b>			310.62 ft SRE		
water resources / environmental consul	tants			7.9 ft SRE			
		IG METHOD:			DEPTH TO WATER: (8/20/2015)		
		' Hollow Stem Auger		7 ft from TOC	28.42 ft below TOC		
LOGGED BY:		ING METHOD:		E STARTED:	DATE COMPLETED:		
RSH	5-π α	continous split barrel	8/1	8/2015	8/18/2015		
Depth (feet) % REC USCS	Graphic Log	Description		-	Vell truction		
0 - 100 CL		TOP SO L, clayey silt with organic matter. LEAN CLAY, silty, light gray with minor ferrous staining, medium sti	ff, dry.		Above ground completion including 2X2 ft concrete pad, four pipe bollards, and locking outer steel casing 32.3 ft of 2 in dia., Sch. 40 PVC solid riser, including 2.7 ft		
4 – 100 ML		S LT, light gray to light brown with ferrous staining and organic mate @ 4.5-5.5 ft, saturated.	erial, soft.		of above ground stick up Cement/bentonite grout from 0 ft bgs to 20.3 ft bgs		
8 - - 100 12 -		FAT CLAY, light gray to gray with ferrous staining, staining increase depth, some organic material, blocky, stiff, moist.	es with		-		
16 — <sup>100</sup> CH		@15 5 ft, with granular calcite concretions.			-		
20 — <sub>100</sub> -		@18-23 ft, fat clay with fine grained sand, stiff, light brown to orang increasing sand content with depth.	e brown,		Bentonite pellet seal from 20.3 ft bgs to 23 ft bgs		
24 – SM 47 GP		S LTY SAND, light brown with a minor amount of clay @ 24-25 ft, color changes to tan, loose, saturated. POORLY GRADED GRAVEL, with silty sand, fine-grained gravel w coarse-grained gravels (<30mm), angular to subrounded, loose, lig			Silica size 10/20 filter pack from 23 ft bgs to 40 ft bgs		
28 – SP	0000	saturated. POORLY GRADED SAND, medium grained, loose, light brown, sat WELL GRADED GRAVEL, fine-to-coarse-grained, rounded to subs fine-to-coarse-grained, including the torus fine-to-coarse-grained, angular to subanglur sand, loose, light brow	angular, with		-		
32 - GW	° ~ 0 0	POORLY GRADED SAND, fine to medium grained, loose, light brows saturated @ 34 ft, orange iron-oxide staining.	1		10 ft of 2 in dia., 0.010 in slot, Sch. 40 PVC screen from 29.6 ft bgs to 39.6 ft bgs		
36 – <sub>n/a</sub> <sub>GW</sub>	0000	WELL GRADED GRAVEL, fine-to-coarse-grained, angular to subro fine-to-medium-grained sand, loose, light brown, saturated.	ounded with		-		
40 —	0 0				- 0.35 ft, 2 in dia., Sch. 40 PVC end cap		
44 —				-	40 ft BOH		
NOTES: Horizontal a	nd vertical ar	ordinates are based on the site referenced as	to sustan				
Tion Lornal a		ordinates are based on the site-referenced coordina were updated to reflect the nomenclature used for the	a anomalan arts	CR Rule network.			

	-		LICCAT Ente DRILLIN McC DRILLIN CME DRILLIN 8.25 SAMPL 5-ft c	itoring Well Installation	MV WEL MV NOR 353 GRO 308 TOT, 42.	L ID: V-8 THING: 31.09 UND SURFACE ELEV.: 3.4 ft SRE AL DEPTH: 6 ft from TOC E STARTED: 8/2015	EASTING: 6472.65 TOC ELEVATION: 311.42 ft SRE DEPTH TO WATER: (8/20/2015) 29.36 ft below TOC DATE COMPLETED: 8/18/2015		
Depth (feet)	% REC	nscs	Graphic Log	Description			truction		
0 4 8 12 16 20 24	n/a 100 78 100 100	CH ML CH		Non-CCR road base, plugged augers to drill through insitu soils. FAT CLAY, light gray, very stiff, dry to moist. @3-3.5 silty clay, minor brown organic material and ferrous staining. SILT, tan to light gray, medium stiff, ferrous staining fine clay laminations (<1mm), moist. FAT CLAY, tan, stiff, with carbonaceous material and ferrous staining, dry to moist. @12 ft, with granular calcite concretions. @17 ft, silty, decreased carbonaceous material. @18 ft, color changes to light gray to gray with iron staining, stiff, moist. @19.8 ft, with grannular calcite concretions. @21.5 ft, occasional fine-grained sand partings, sa yellowish orange, clay has iron oxide staining in veri fractures. @23 ft, color changes to brown, soft, moist to very @26.8 ft, sandy clay, sand is fine-grained, saturate	orange , with d oxide nd is ical noist.		Above ground completion including 2X2 ft concrete pad, four pipe bollards, and locing outer steel casing 32.2 ft of 2 in dia., Sch. 40 PVC solid riser, including 3.0 ft of above ground stickup Cement/bentonite grout from 0 ft bgs to 25 ft bgs Bentonite pellet seal from 25 ft bgs to 26.9 ft bgs		
28 — - 32 — -	44	GW		WELL GRADED GRAVEL with sand, clayey in uppe fine- to coarse-grained gravel (less than 2"), fine- to coarse-grained sand, gravel is rounded to subangul is subangular to angular, loose, brown, saturated, cl gravel.	ar, sand		Silica size 10/20 filter pack from 26.9 ft bgs to 39.4 ft bgs 10 ft of 2 in dia., 0.010 in slot, Sch. 40 PVC screen from 29.2		
36 - - 40 - -	n/a		0 0 0 0 0 0 0 0 0 0 0				ft bgs to 39.2 ft bgs 0.35 ft, 2 in dia., Sch. 40 PVC end cap 39 ft BOH		
44 – NOTES				pordinates are based on the site-referenced coordinates were updated to reflect the nomenclature used for the	5-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6-6				

Associates Ltd, water resources / environmental consultants	PROJECT: Monitoring Well Installation LOCATION: Entergy Independence Landfill DRILLING CONTRACTOR: McCray Drilling, LLC DRILLING EQUIPMENT: CME 750X DRILLING METHOD: 8.25" Hollow Stem Auger SAMPLING METHOD: 5-ft continous split barrel Description	307.9 ft SR TOTAL DEPTH: 47.8 ft from	MW-9           WELL ID: MW-9           NORTHING:         EASTING:           3059.28         5967.5           GROUND SURFACE ELEV.:         TOC ELEVATION:           307.9 ft SRE         310.39 ft SRE           TOTAL DEPTH:         DEPTH TO WATER: (8/20/20'           47.8 ft from TOC         28.28 ft below TOC           DATE STARTED:         DATE COMPLETED:	
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	Non-CCR road base, plugged augers to drill through to insitu         FAT CLAY with silt, light gray, very stiff, with carbonaceous m concretions, moist to dry.         CLAYEY SAND, fine- to medium-grained, light gray, loose, wit material, moist.         FAT CLAY with increasing silt content with depth, light gray, loose, with calcitic concretions and carbonaceous material, orange a staining in vertical fractures.         SLT, light gray with orange and black ferrous staining, soft, m @ 17.5 ft, with clay and minor very fine-grained sand.         @ 18 ft, very moist.         @ 20.7 ft, saturated.         FAT CLAY, light gray to light brown, stiff to very stiff, moist.         @ 24.7 ft color changes to dark gray with minor silt partings, n         @ 28 ft, color changes to gray, soft.	aterial and calcite th carbonaceous ery stiff, moist, ind black ferrous oist. ttter.		Above ground completion including 2X2 ft concrete pad, four pipe bollards, and locking outer steel casing 37.4 ft of 2 in dia., Sch. 40 PVC solid riser, including 2.5 ft of above ground stickup Cement/bentonite grout from 0 ft bgs to 29.5 ft bgs
$ \begin{array}{cccccccccccccccccccccccccccccccccccc$	WELL GRADED GRAVEL with fine- to medium-grained sand, coarse-grained chert gravel less than 30 mm, subrounded to brown to yellowish orange, saturated. WELL GRADED SAND, fine- to coarse-grained sand, loose, if saturated     WELL GRADED GRAVEL, fine- to coarse-grained, subangul guartz/ chert gravel grain size fines upwards saturated. WELL GRADED SAND, fine to coarse sand, subrounded to a WELL GRADED GRAVEL, fine- to coarse-grained (less than medium-grained sand, subrounded to subangular, quartz/ che saturated.	angular, light ight brown, ar to angular, ngular, saturated. 3"), with		Bentonite pellet seal from 29.5 ft bgs to 31.5 ft bgs Silica size 10/20 filter pack from 31.5 ft bgs to 45 ft bgs 10 ft of 2 in dia., 0.010 in slot, Sch. 40 PVC screen from 34.9 ft bgs to 44.9 ft bgs 0.35 ft, 2 in dia., Sch. 40 PVC end cap

RSH (	DRILLING EQUIPMENT: TS 150 Rig #1154 DRILLING METHOD: Sonic with 4x6 in dia. core and case SAMPLING METHOD: Continuous with 10 ft, 4 in dia. core barrel	4326.9 GROUND SURFACE ELEV.: 310.7 ft SRE TOTAL WELL DEPTH: 49.7 ft below TOC DATE STARTED: 2/13/2017	EASTING: 5583.2 TOC ELEVATION: 313.63 ft SRE DEPTH TO WATER: 3/6/2017 32.85 ft below TOC DATE COMPLETED: 2/14/2017
Depth (feet) % REC USCS Graphic	Description	Constr	
$ \begin{array}{c} 0 \\ - \\ 75 \\ 75 \\ CH \\ CL \\ CL$	<ul> <li>FILL, limestone gravel road base material.</li> <li>FAT CLAY, light grey to tan with organic material, med stiff, moist, silty upper 1 ft.</li> <li>LEAN CLAY, silty, light grey with orange ferrous stainin and organic material, stiff, moist.</li> <li>SILT with clay, light grey to light brown with organic material and ferrous staining, soft, moist.</li> <li>CLAY with silt, light grey with orange ferrous staining, blocky, stiff, moist.</li> <li>@24-26 ft with increased ferrous staining.</li> <li>@26-28 ft sandy, very fine-grained, light brown, soft, w</li> <li>FAT CLAY, light grey with ferrous staining, stiff, moist.</li> <li>@20-31 ft sandy, sand is fine-grained, grey, very moist soft.</li> <li>POORLY GRADED GRAVEL with sand, fine to coarse-grained, sand is fine to medium-grained, brown loose, saturated.</li> <li>@36-46 ft with minor silt content, fine to coarse grained-sand, fine to medium-grained subrounded che gravel.</li> <li>@46-47 ft with coarse-grained subangular sand and gr (&lt;1 in)</li> <li>Boring terminated at 47 ft.</li> <li>Soil descriptions are based on the soil log from 710D located 30 ft from 7105. Soils were continuously samp and logged at 710D using a 4-in core barrel. Samples 710S were visually observed to verify that 710S was advanced in similar soils logged for 710D.</li> </ul>	ret. t, t, tavel led	<ul> <li>four pipe boliards, and locking outer aluminum casing</li> <li>Concrete from 0 ft to 2.0 ft</li> <li>39.5 ft of 2 in dia., Sch. 40</li> <li>PVC solid riser, including 2.9 ft of stickup and a 0.12 ft PVC coupler for dedicated pump mount.</li> <li>Cement/bentonite grout from 2.0 ft to 26.0 ft</li> <li>Bentonite pellet seal from 26.0 ft to 30.0 ft</li> <li>Silica size 10/20 filter pack from 30.0 ft to 47.0 ft</li> <li>10.0 ft of 2 in dia., 0.010 in slot, Sch. 40 PVC screen</li> <li>0.25 ft, 2 in dia., Sch. 40 PVC end cap</li> </ul>

5			PROJE	ECT:	BOF	RING ID:	
			Moni	itoring Well Installations	MW	/-11	
A		Sector Contraction of the sector of the sect	The approximation of the appro		WELL ID:		
=	<u> </u>		Ente	rgy Independence Landfill	MW	/-11	
_		1	8	NG CONTRACTOR:	NOF	RTHING:	EASTING:
	3			ade Environmental	439	2.0	6005.9
			DRILLI	NG EQUIPMENT:	Westerney	OUND SURFACE ELEV .:	TOC ELEVATION:
-	Asso		11.1	probe 8140LC	00000000	0.0 ft SRE	313.25 ft SRE
water resour	ces / env[ron	mental consult	ante	NG METHOD:		TAL WELL DEPTH:	DEPTH TO WATER: 3/6/2017
			40050 H. H. H.	c with 4x6 in dia. core and case	Care reaction	0 ft below TOC	
-			00%042005				32.51 ft below TOC
LOGGE	D BY:			LING METHOD:		DATE STARTED: DATE COMPLETED: 2/23/2017 2/23/2017	
RSH		-	Cont	inuous 5 ft and 10 ft, 4 in dia. core barrel	ZIZ	5/2017	2/23/2017
feet	REC	S	<u>ic</u>	54°-45 83 8556		V	/ell
th	% RI	nscs	Graphic Log	Description			ruction
Depth (feet)	0,	2	бд			0013	luction
						_	
							Above ground completion including 3x3 ft concrete pad,
0 —	ä	3		NO RECOVERY, limestone gravel road base materia	l at		four pipe bollards, and locking
	0			surface.			outer aluminum casing
_	5	8		SILT with clay, light grey with orange ferrous staining	and	- 🕅 🕅 -	Concrete from 0 ft to 2.0 ft
	60	ML		brown organic material, medium stiff, moist.	anu	$\otimes$	
10 —		IVIL		@7-10 ft with decreased clay content, with minor fine-grained sand content, soft, very moist to wet.			38.8 ft of 2 in dia., Sch. 40 PVC solid riser, including 3.3 ft
10	00			@10-11 ft clayey.			of stickup and a 0.12 ft PVC
	90			LEAN CLAY, silty, light grey with orange ferrous stain blocky, stiff, moist.	ning,		coupler for dedicated pump mount
-	8		//////	@15-20 ft medium stiff, moist to very moist.			mount
	85	CL	//////				Cement/bentonite grout from
20 —	i.		//////	@20 ft sandy, with reddish brown ferrous staining, so	ft,		20 2.0 ft bgs to 20.5 ft
	100		//////	increasing sand content with depth.			Bentonite pellet seal from 20.5
		GC	10/0/0	CLAYEY GRAVEL with sand, fine-to coarse-grained, is medium to coarse-grained, brown, loose, saturated			ft bgs to 25.0 ft
		00	0/0/0	@26.5 ft gravely fat clay lens (0.5 ft), clay is grey, gra			-
	Narah		//////	fine-grained, wet, soft. SANDY LEAN CLAY, grey, sand is very fine-grained,	aaft	-	Silica size 10/20 filter pack 30 ▼ from 25.0 ft bgs to 45.0 ft
30 —	50	CL	//////	very moist.	Soit,		30 10.0 11 20.0 11 200 10 10.0 11
			//////				
-	ŝ	2	<del>/////////////////////////////////////</del>	CLAYEY SAND, fine to medium-grained, medium der	nse.	-	10.0 ft of 2 in dia., 0.010 in
		SC		saturated, increasing clay content with depth.			slot, Sch. 40 PVC screen
40 -	50		10/0	CLAYEY GRAVEL with sand, fine to coarse chert, sa	nd is		40
1000	Dimension of	GC	0/0/0	fine to medium-grained, clay is fat and grey, wet to saturated.			0.03
			/0/0/	Saturateu.			0.20 ft, 2 in dia., Sch. 40 PVC
1	X	2		Boring terminated at 45 ft.			end cap
							19205
50 —	÷.					-	50
-	á					-	
60 —	3						60
	2						
70 —	÷.					-	70
_	ä					_	
	NOTES: Borehole and/or well IDs were updated to reflect the nomenclature used for the EPA CCR Rule network.						
Horizontal and vertical data are based on the Harmon Surveying report dated April 10, 2017 (site referenced coordinate system). SRE=Site referenced elevation.							

**Geotechnical Data** 

EPA CCR Well ID on Site Map	Well ID on Geotechnical Data Test Forms <sup>(a)</sup>
MW-1R <sup>(b)</sup>	701S-R, 701M, 701D <sup>(b)</sup>
MW-3 <sup>a</sup>	703S, 703M, 703D <sup>(b)</sup>
MW-6	706S
MW-7	707S
MW-8	708S
MW-9 <sup>(b)</sup>	709S, 709M, 709D <sup>(b)</sup>
MW-10 <sup>(b)</sup>	710S, 710M, 710D <sup>(b)</sup>
MW-11	711S

#### ENTERGY INDEPENDENCE PLANT WELL ID NUMBER KEY.

Notes:

a. Geotechnical soil samples were collected and tested using well IDs associated with the landfill's ADEQ solid waste permit (Permit No. 0200-S3N-R2).

b. Well cluster consisting of three closely spaced wells with different depths. Due to scale, these are represented as one well on the site map.



#### Measurement of Hydraulic Conductivity

Client: FTN

Date of Report: 06/07/01

**Job #:** E-5-672

Project Name: Entergy/Independence Plant

Sample I.D.: Composite Sample Boring E-10

Soil Description: Brown Clay/Remolded Sample

Test Media: City of Memphis Public Water Supply

	<u>Pre-Test</u>
Wet Density	122.1 lbs/ft <sup>3</sup>
Dry Density	99.5 lbs/ft <sup>3</sup>
Moisture (% Dry Wt)	22.7%
Porosity (n) Total	.372
Initial Degree of Saturation	92.3%
Percent Compaction	95.9%
Deviation from Opt. Moisture	+3.1%
"B" Coefficient (post saturation)	1.00
Range of Hydraulic Gradient	20.1-34.8

**Permeability** 

**Temperature Correction**,  $R_r = 1.002$ 

$$\begin{split} &K_1 = 3.7 \ X \ 10^{.9} \ cm/sec \\ &K_2 = 3.4 \ X \ 10^{.9} \ cm/sec \\ &K_3 = 3.5 \ X \ 10^{.9} \ cm/sec \\ &K_4 = 3.6 \ X \ 10^{.9} \ cm/sec \end{split}$$

Coefficient of Permeability,  $K_{20}$  = 3.6  $\rm X~10^{.9}~cm/sec$ 

Tested in accordance with ASTM D-5084-90.

Lab No. P-01-034

Reviewed By:

David D. McCray

6756 Buckles Cove Memphis, TN 38133 901-385-1199 fax 901-386-6614



#### Measurement of Hydraulic Conductivity

Client: FTN

Date of Report: 06/07/01

**Job #:** E-5-672

Project Name: Entergy/Independence Plant

Sample I.D.: Boring E-2 Shelby Tube, Depth 12'-14'

Soil Description: Brown Clay

Test Media: City of Memphis Public Water Supply

	<u>Pre-Test</u>
Wet Density	111.5 lbs/ft <sup>3</sup>
Dry Density	81.3 lbs/ft <sup>3</sup>
Moisture (% Dry Wt)	37.2%
Porosity (n) Total	.501
Initial Degree of Saturation	93.2%
"B" Coefficient (post saturation)	1.00
Range of Hydraulic Gradient	16.0-31.3

**Permeability** 

Temperature Correction,  $R_t = 1.004$ 

$$\begin{split} \mathbf{K_1} &= 2.0 \ \mathrm{X} \ 10^{-8} \ \mathrm{cm/sec} \\ \mathbf{K_2} &= 1.5 \ \mathrm{X} \ 10^{-8} \ \mathrm{cm/sec} \\ \mathbf{K_3} &= 1.3 \ \mathrm{X} \ 10^{-8} \ \mathrm{cm/sec} \\ \mathbf{K_4} &= 1.8 \ \mathrm{X} \ 10^{-8} \ \mathrm{cm/sec} \end{split}$$

Coefficient of Permeability,  $K_{20} = 1.7 \times 10^{-8} \text{ cm/sec}$ 

Tested in accordance with ASTM D-5084-90.

Reviewed By: David D. McCray

Lab No. P-01-027

6756 BUCKLES COVE MEMPHIS, TN 38133 901-385-1199 FAX 901-386-6614



Measurement of Hydraulic Conductivity

**Client:** FTN

Date of Report: 06/07/01

Job #: E-5-672

Project Name: Entergy/Independence Plant

Sample I.D.: Boring E-9 Shelby Tube, Depth 16'-18'

Soil Description: Brown Clay

Test Media: City of Memphis Public Water Supply

	<u>Pre-Test</u>
Wet Density	114.6 lbs/ft <sup>3</sup>
Dry Density	84.7 lbs/ft <sup>3</sup>
Moisture (% Dry Wt)	35.3%
Porosity (n) Total	.480
Initial Degree of Saturation	94.7%
"B" Coefficient (post saturation)	1.00
Range of Hydraulic Gradient	18.7-34.8

**Permeability** 

Temperature Correction,  $R_t = 1.002$ 

 $K_1 = 6.7 \times 10^{.9} \text{ cm/sec}$   $K_2 = 7.1 \times 10^{.9} \text{ cm/sec}$   $K_3 = 7.2 \times 10^{.9} \text{ cm/sec}$   $K_4 = 7.1 \times 10^{.9} \text{ cm/sec}$ 

Coefficient of Permeability,  $K_{20} = 7.0 \times 10^{-9} \text{ cm/sec}$ 

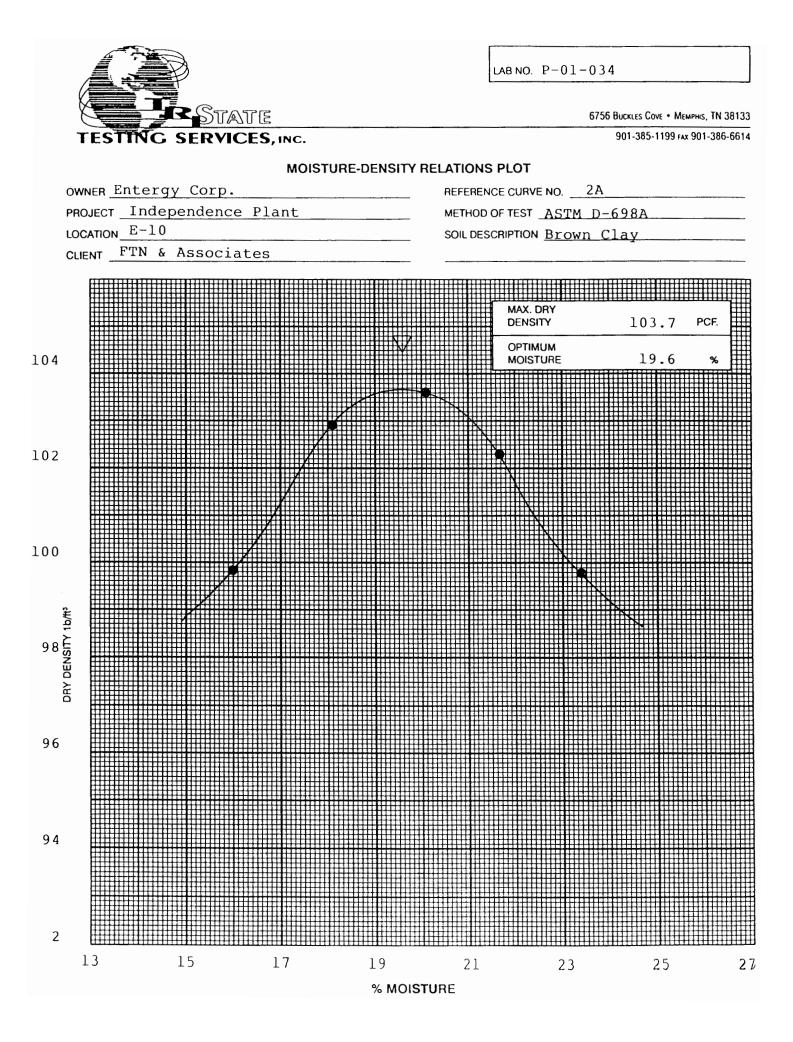
Tested in accordance with ASTM D-5084-90.

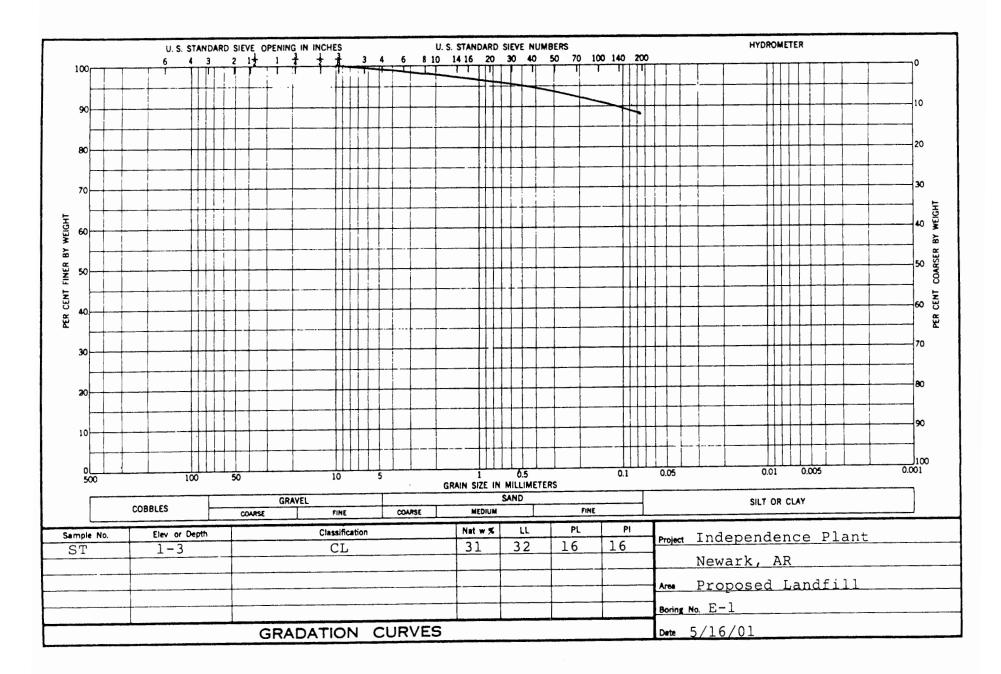
Lab No. P-01-031

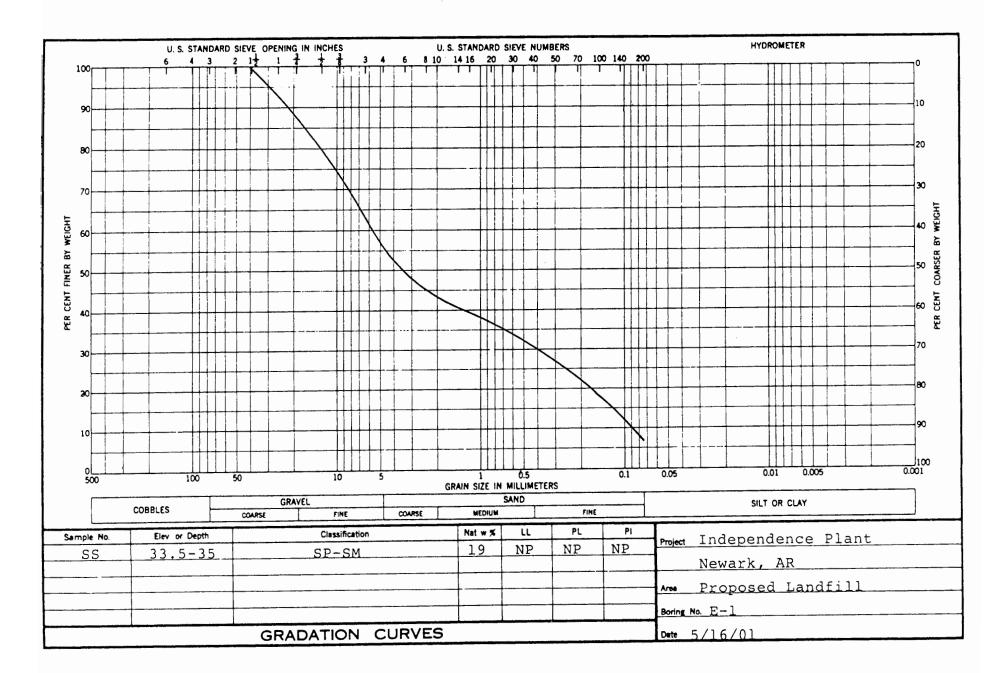
Reviewed By:

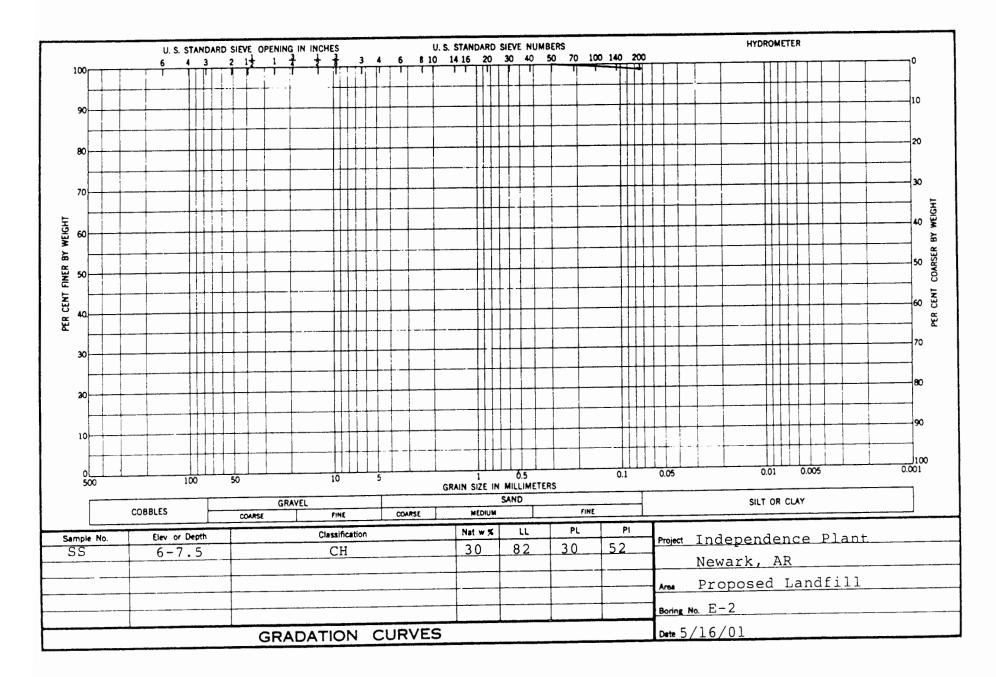
David D. McCray

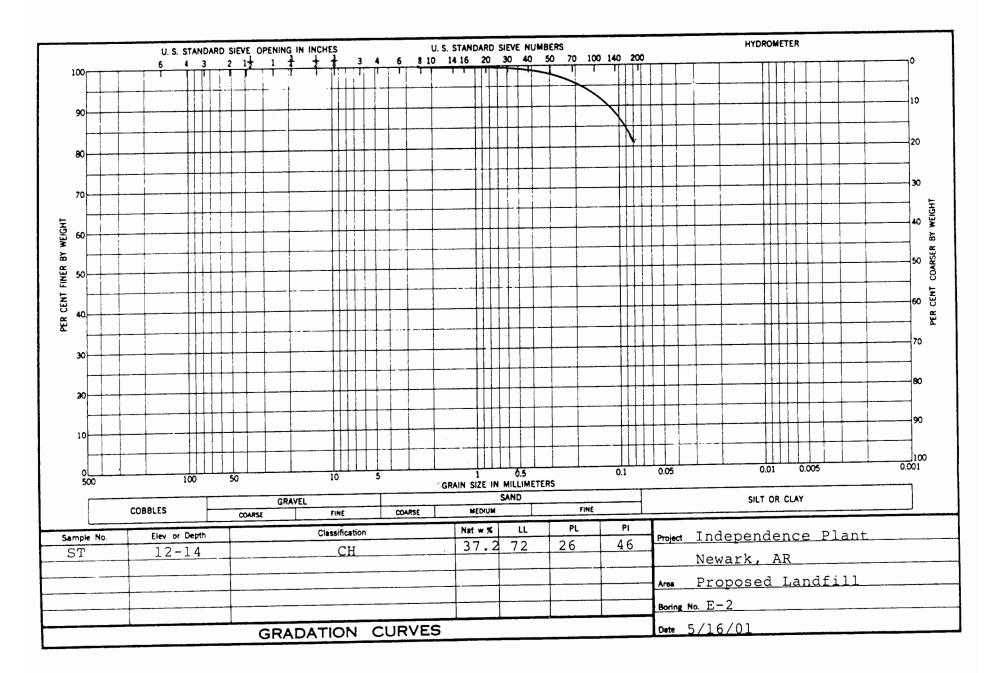
901-385-1199

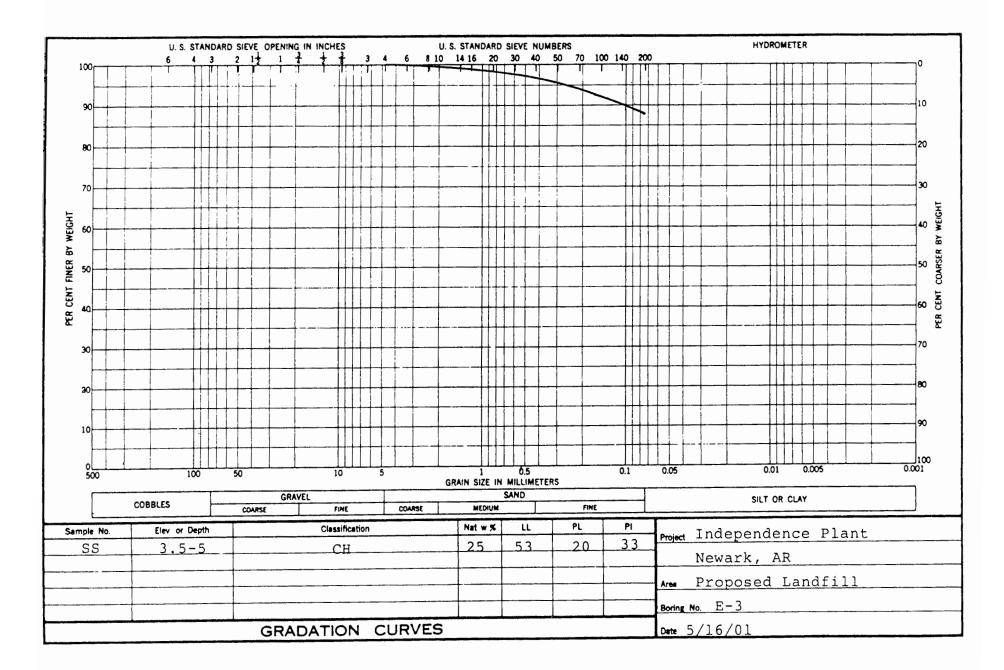


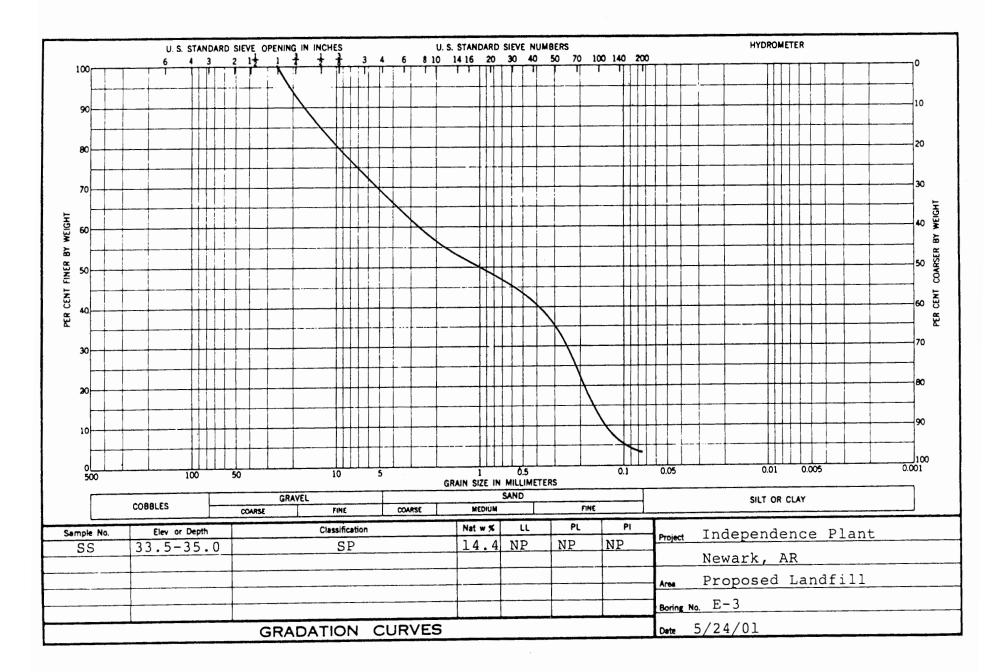


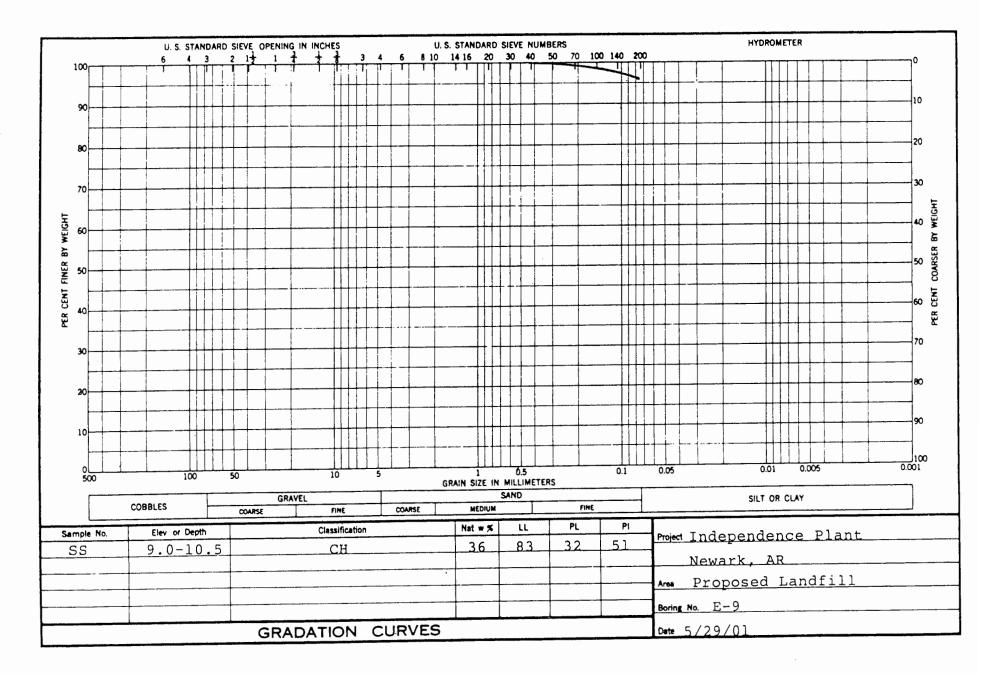


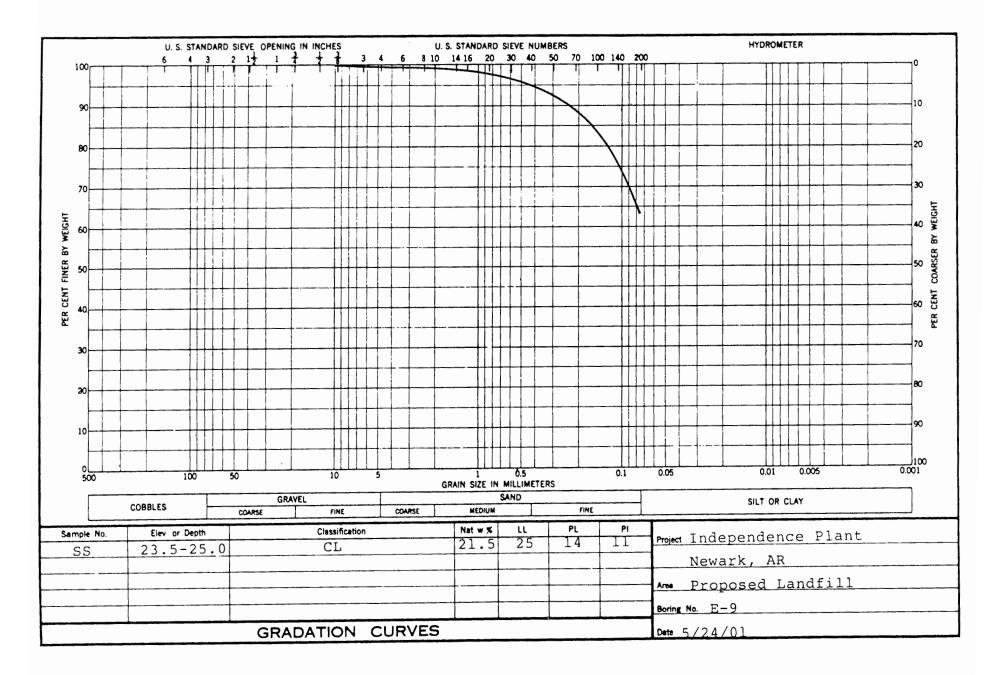


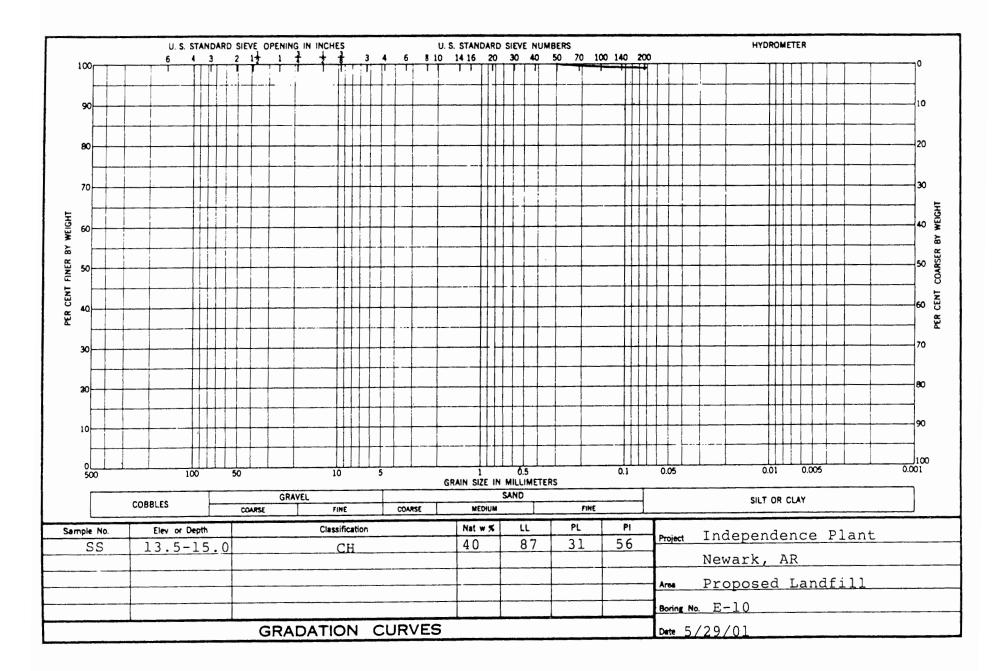












# Unconsolidated, Undrained (Q) Compressive Strength Of Cohesive Soils In Triaxial Compression AASHTO T 296-94 / ASTM D 2850

Tri State	Testing	Services	Memphis,	Tennessee
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A	Top Dia.	7.2619 cm			Before	After
April 17, 2001	Mid Dia.	7.2619 cm			Test	Test
	Bot Dia.	7.2619 cm		Tare No.		120.00
Job No. 1182	Avg Dia.	7.2619 cm		Wet Wt.+Tare		170.24
Boring No. E-3	Area	41.418 cm <sup>2</sup>		Dry Wt.+Tare		132.78
Depth 15.5 ft.	Height	14.585 cm		Tare Weight		31.19
Sample No. n/a	Wet Weight	1098.70 gm	HHK	Wt. Water		37.46
Specimen No.	Spec. Grav.	2.71		Wet Soil		139.05
Test No.	Wet Density	113.55 pcf		Dry soil		101.59
	Dry Density	82.96 pcf	1	Water Content		36.87
	Void Ratio	1.039		Confining pressure	: 12 psi	
Material:	Saturation	96.142	Failure Sketch	Rate of strain: 1.2	7228 mm/mi	nute

Stiff light gray clay (CH), slightly silty, slickensided and friable

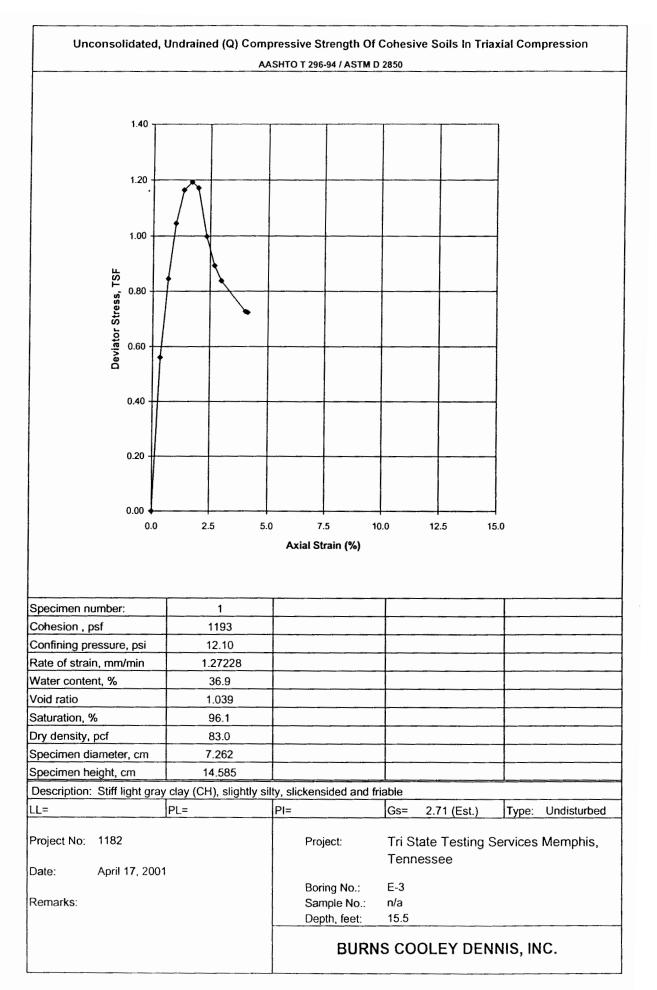
	Elapsed	Dial	Dial	Axial		Corrected	Axial	Deviator	
	Time	Reading	Reading	Strain	1-Strain	Area	Load	Stress	Cohesion
	hh:mm	mm	.001 in.	in/in		cm <sup>2</sup>	lbs	tsf	psf
	0:00	0.000	0.000	0.0000	1.000	41.42	0.00	0.00	0
_ [	0:00	0.519	20.436	0.0036	0.996	41.57	50.20	0.56	561
1	0:00	1.006	39.623	0.0069	0.993	41.71	75.88	0.85	845
- [	0:01	1.474	58.043	0.0101	0.990	41.84	94.18	1.05	1046
[	0:01	1.971	77.585	0.0135	0.986	41.99	105.18	1.16	1164
[	0:02	2.477	97.525	0.0170	0.983	42.13	108.16	1.19	1193
[	0:02	2.888	113.686	0.0198	0.980	42.25	106.56	1.17	1172
[	0:02	3.443	135.547	0.0236	0.976	42.42	91.11	1.00	998
[	0:03	3.958	155.833	0.0271	0.973	42.57	81.80	0.89	893
	0:03	4.383	172.551	0.0301	0.970	42.70	77.02	0.84	838
[	0:04	5.903	232.416	0.0405	0.960	43.17	67.48	0.73	726
Ī	0:04	6.043	237.927	0.0414	0.959	43.21	67.27	0.72	723
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Tested By: H

Computed By:

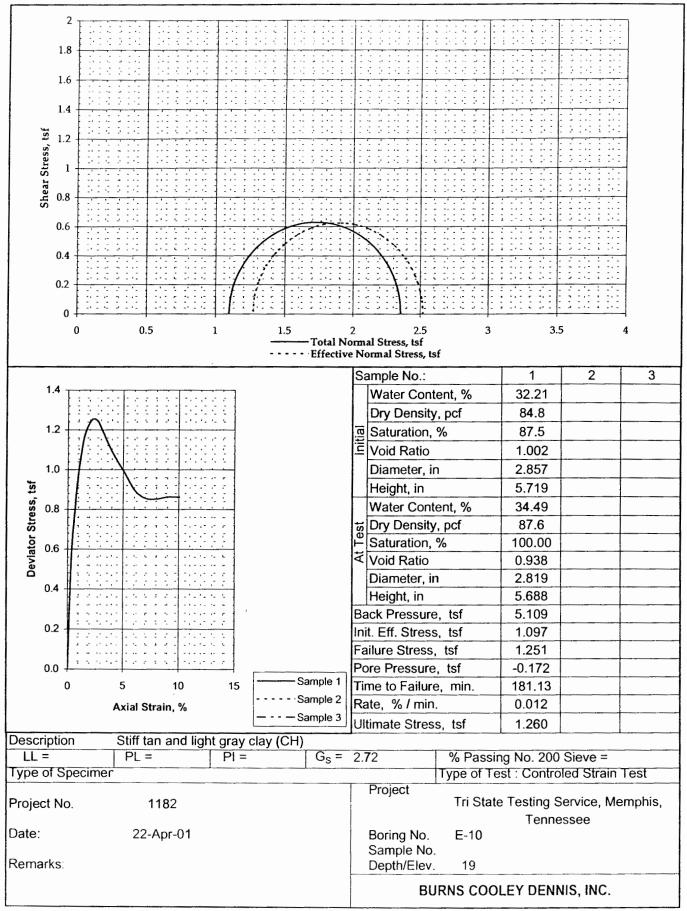
Checked By

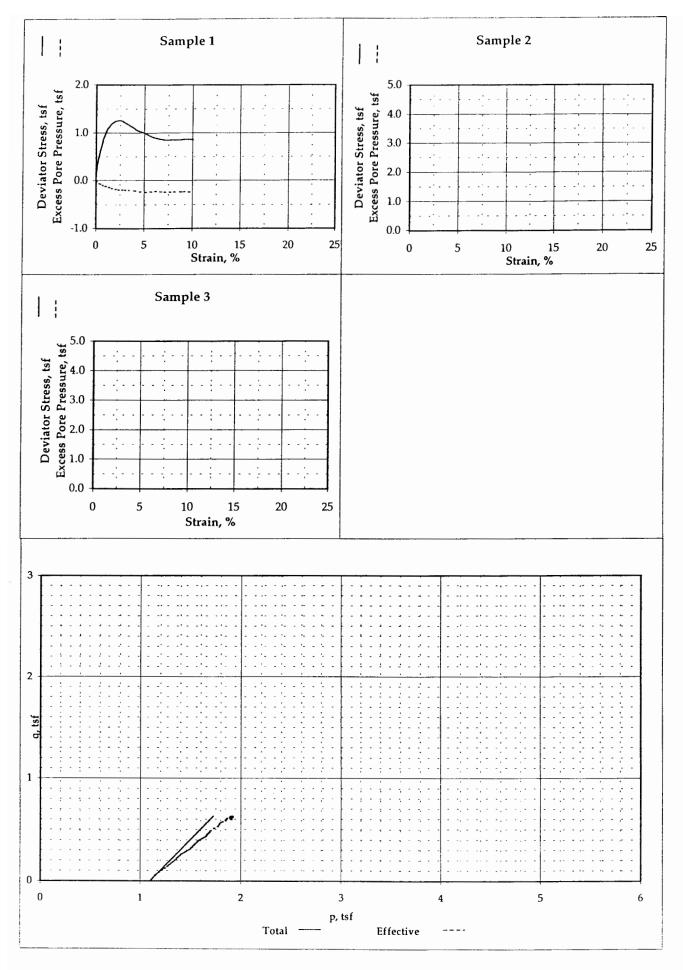
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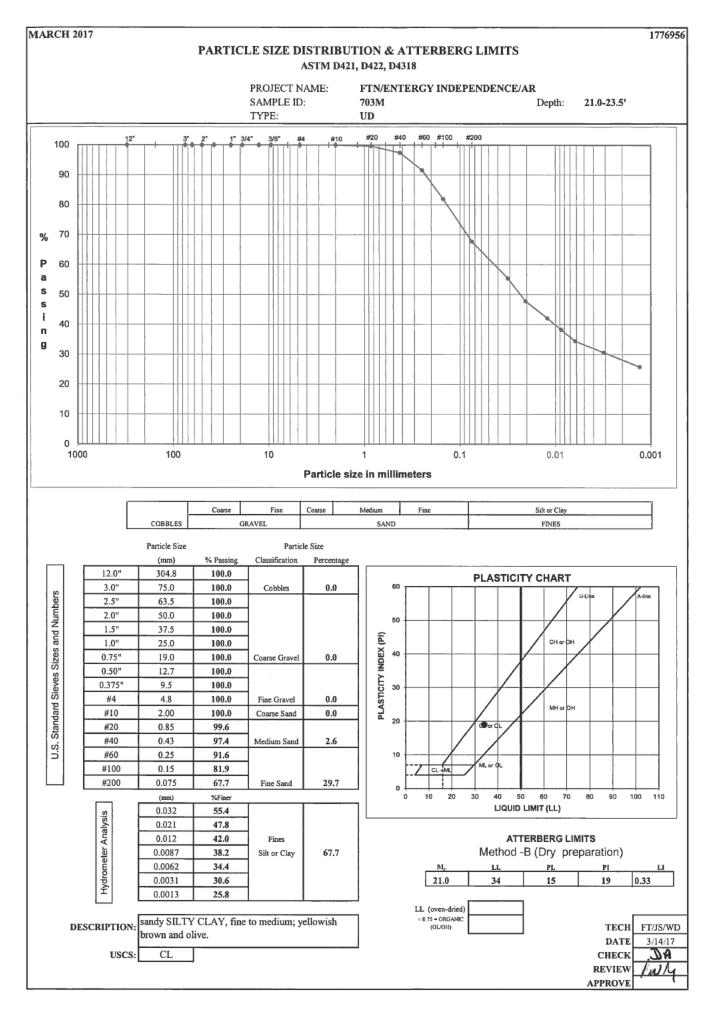
Standar	d Test M	lethod for C			drained (R) Tr )		-	ion Test for C	ohesive	Soils
					Service, Mem					
		Be	fore Tes	÷		t Test				
	0000	Top Dia.	7.2568	cm	Delta H	0.08	cm		Before	After
August 30	, 2000	Mid Dia.	7.2568	cm	Delta Vol	-18.60	mi	1	Test	Test
		Bot Dia.	7.2568	cm				Tare No.	151	
Job No.	1182	Avg Dia.	7.2568	cm				Wet Wt.+Tare	217.80	
Boring No.	E-10	Area	41.360		Area	40.265	cm <sup>2</sup>	Dry Wt.+Tare	172.14	
Depth 0 ft.	19	Height	14.526		Height	14.446		Tare Weight	30.38	
Sample No.		Wet Weight			Wet Weight	1097.90		Wt. Water	45.66	
Specimen No.		Spec. Grav.	2.72	-	Water Content		3	Wet Soil	187.42	
Test No.		Wet Density			Wet Density	117.8	ocf	Dry soil	141.76	
		Dry Density	84.8	•	Dry Density	87.6		Water Content	32.21	
		Void Ratio	1.002	•	Void Ratio	0.938	poi	Confining pres		osi
Material:		Saturation	87.451		Saturation	100.0%		Rate of strain:		
	ight grou		07.401		Gaturauon	100.078			0.017201	
Stiff tan and I	ight gray	ciay (CH)								
Elapsed	Pore	Change In	Dial	Dial	Axial		Corrected	Axial	Deviator	
Time	Pressure	Pore Pres.	Reading	Reading	Strain	1-Strain	Area	Load	Stress	
hh:mm	psi	psi	mm	.001 in.	in/in		cm <sup>2</sup>	lbs	tsf	
0:00	70.95	0.00	0.000	0.000	0.0000	1.000	41.36	0.00	0.00	
0:30	70.10	-0.85	0.494	19.457	0.0034	0.997	41.50	48.12	0.54	
0:59	69.62	-1.33	0.983	38.720	0.0068	0.993	41.64	71.09	0.79	
1:24	69.31	-1.65	1.398	55.034	0.0096	0.990	41.76	87.11	0.97	
1:56	69.02	-1.93	1.955	76.965	0.0135	0.987	41.92	101.29	1.12	
2:21	68.69	-2.27	2.365	93.114	0.0163	0.984	42.04	107.59	1.19	
2:54	68.45	-2.50	2.923	115.069	0.0201	0.980	42.21	112.54	1.24	
3:19	68.34	-2.61	3.341	131.529	0.0230	0.977	42.33	114.44	1.26	
3:52	68.29	-2.67	3.916	154.177	0.0270	0.973	42.51	113.94	1.25	
4:17	68.22	-2.73	4.345	171.057	0.0299	0.970	42.64	111.37	1.21	
5:39	68.06	-2.89	5.760	226.788	0.0397	0.960	43.07	101.33	1.09	
7:14	67.59	-3.37	7.387	290.834	0.0509	0.949	43.58	92.73	0.99	
8:36	67.88	-3.08	8.839	347.994	0.0608	0.939	44.04	84.65	0.89	
9:58	67.62	-3.34		403.450	0.0705	0.929	44.50	81.85	0.85	
11:20	67.74	-3.21	11.693		0.0805	0.920	44.98	82.56	0.85	
12:43	67.75	-3.21	13.156		0.0906	0.909	45.48	84.54	0.86	
14:05	67.51	-3.44	14.589	574.351	0.1004	0.900	45.98	85.26	0.86	
Tested By:	H		Comp	uted By:	н			Checked By:	2 A	

## Standard Test Method for Consolidated Undrained (R) Triaxial Compression Test for Cohesive Soils AASHTO T 297 / ASTM D 4767-95



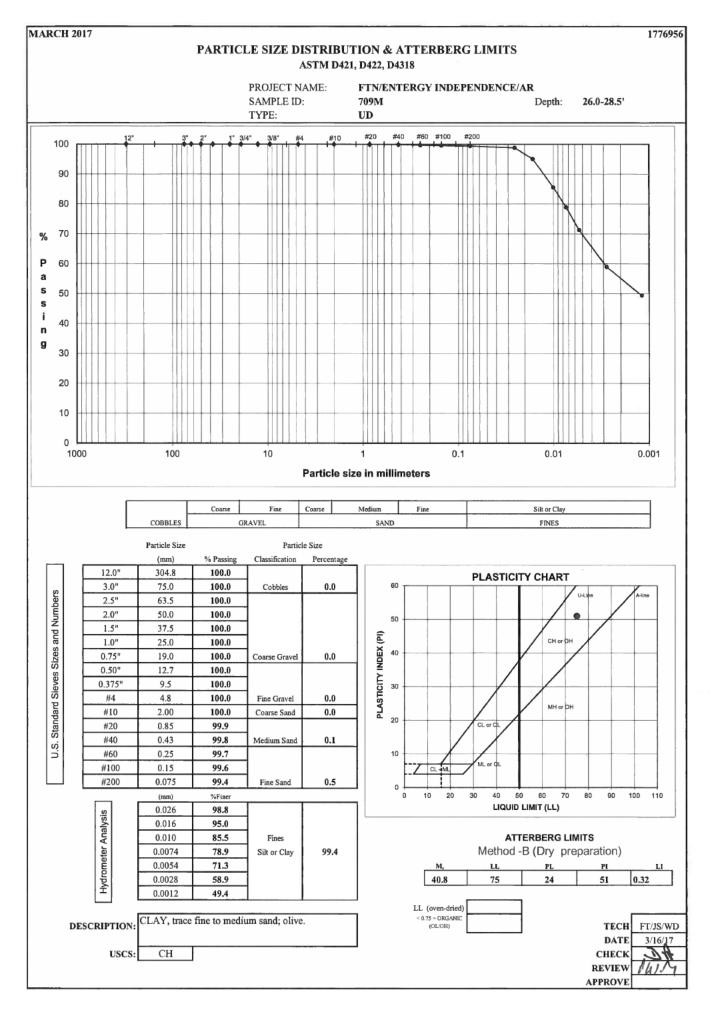


Burns Cooley Dennis, Inc. 551 Sunnybrook Rd. Ridgeland, MS 39157



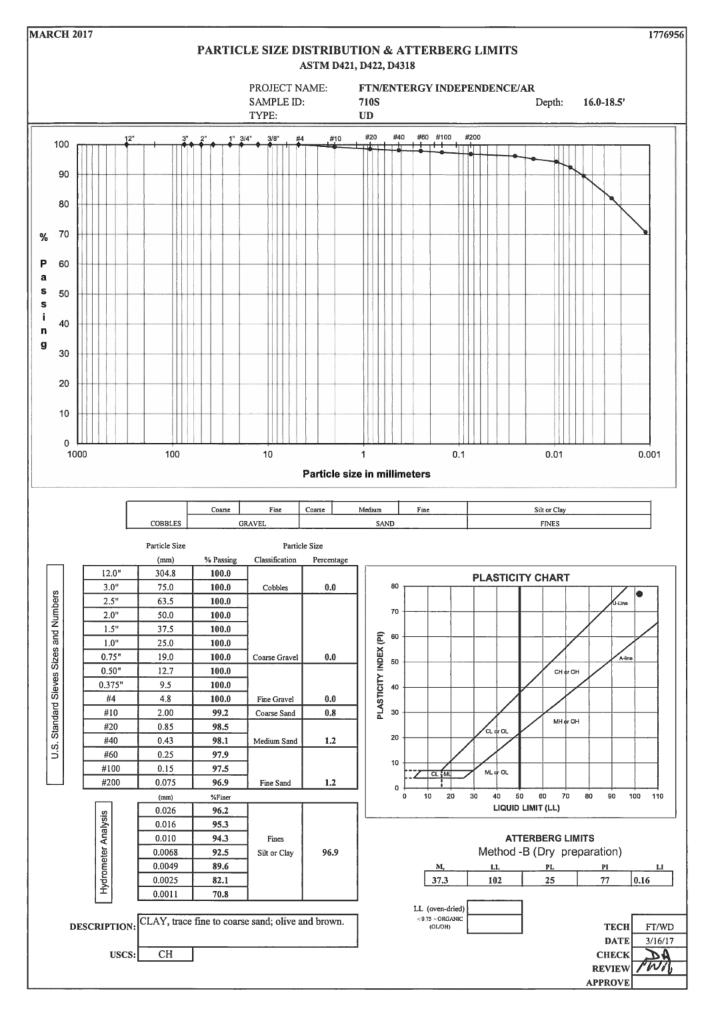
							A	STM D 5	084					
				MB	ETHOD C	, FALLIN	G HEAD	W/INCRE	ASING TA	AIL WAT	ER PRESS	SURE		
Outflow Rate 0.000223 Outflow/Inflow Ratio 0.91 CHECK														
1				21.0	22.51	4								
SASTEND 5084           METHOD C, FALLING HEAD W/INCREASING TAIL WATER PRESSURE           ROJECT TITLE FUNCENCE/AR PROJECT NUMBER FI770956         Colspan="2">Colspan="2">Coll Press Oaly Using Pipettes & Burettes Data SAMPLE TYPE         Colspan="2">Coll Press Oaly Using Pipettes & Burettes Data Sample Data, Initial Height, inches         Sample Data, Initial Height, inches         Sample Data, Initial Height, inches         Sample Data, Final Height, inches         Value (0.7)           Sample Data, Initial Height, inches         Sample Data, Final Height, inches         Sample Data, Final Height, inches         Using Pipettes Oaly Using Pipettes Oaly Using Pipettes Oaly Using Pipettes Oaly Using Pipettes Oaly Oals         Water Contents         Initial Final Final Final Height, inches           Sample Data, Final Height, inches         Sample Data, Final Height, inches         Sample Data, Final Pipettes Oaly Oals         Wit solikare, f.         Initial S48.65         Final Gis3.88           Water Content, %         2010           Pipet Sample Data, Final Height, inches         Sample Data, Final Height, inches           Water Content, %         Oalspan="2">Content         Sample Data, Final Height, inches           Value (min Sample Data, Final Height, inches         Sample Data, Final Height, inches														
							5							
Sample Da	ta, Initial													
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						psı		n^3						
1	2			-				antont 0/			-			
						{					water Con	tent	21.00%	21.30%
						{		y, pci		DESC	DIBTION	and SUT	VCLAV 6	na ta madinan
						{		ma par (aa)		DESC	.KIF HUN			
					15.00			÷ · ·				yenowish u	nown and on	IVC.
							Outriow voi	unie per (cc)	1.00		USCS	CL		
Saturation		72.070				PERM	/FANT · D	eaired Tan V	Water		0505		1	
						- I LIG	BERTIN B							
TIN	<b>IE FUNCT</b>	ON	REAL	DINGS		TIME	E IN MINUT	ES & SECO	ONDS			VOL	UME	PERMEABILITY
DATE	HOUR	MIN	Inflow	Outflow	Temp.	dt	dt	dt, acc	Head	(H1/H2)	Gradient	Inflow	Outflow	@ 20 Degrees C
			(cc)	(cc)		(min)	(sec)	(sec)	(cm)	(inc.)		· · /	(cc)	(cm/sec)
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03/16/17	10	26	9.1	16.1	19.7	20.0	1200	75780	217.90	1.01	27.47	1.00	1.00	7.3E-07 *
	Inflow Rat	e	0.000244								*PE	RMEABIL	ITY REPO	RTED AS 7.8E-07 cm/sec
	Outflow R	ate												
	Outflow/In	flow Ratio	0.91											
														CHECK JA
														REVIEW /W/Y
													A	APPROVE

### GOLDER ASSOCIATES INC.



			MI	THOD		· A	ASTM D 5				TIRE					
PROJECT TITLI PROJECT NUMI SAMPLE ID	BER 1776956 709M	ERGY IND	EPENDENO 26.0-	CE/AR	Usir BOARD#	Using Pip g Pipettes & 6	ettes Only & Burettes TECH	YES NO SDM/PWM		IL WATER PRESSURE COMMENTS						
SAMPLE TYPE	UD				CELL #	6	DATE	3/15/17								
Sample Data, Init	al															
Height, inches	3.120	]			_	Sample Da	ta, Final			Water Con		Initial	Final			
Diameter, inches	2.842	1	B-Value,f	0.99		Height, incl		3.092		Wt soil&ta		574.80	580.32			
Area, cm^2	40.93	4	Cell Pres.	72.0	psi	Diameter, i		2.792		Wt soil&ta	re, f	408.36	416.50			
Volume, cm^3	324.33	4	Bot. Pres.	53.0	psi	Area, cm <sup>2</sup>		39.50		Wt Tare		0.00	8.43			
Mass, g	574.80	4	Top Pres.	50.0	psi	Volume, cn	n^3	310.21		Wt Moistu		166.44	163.82			
Moisture Content, % 40.8 Dry Density, pcf 78.6		4	Head, cm	211.02	-	Mass, g	ontont 0/	572.30		Wt Dry So		408.36	408.07			
		-	Max. Grad. Min. Grad.	29.98 29.30	-	Moisture C		40.15 82.14		Water Con	tent	40.76%	40.15%			
Spec. Gravity (assun	,	-	Min. Grad. Max. E.S.	29.30	-	Dry Density Saturation	y, per	82.14	DESC	RIPTION	CLAV tree	e fine to ma	dium sand; olive.			
		-	Min. E.S.	19.00	1	Inflow Volu	ime per (cc)		DESC		CLAI, Had	le fine to met	anum sana, onve,			
Void Ratio	<sup>3</sup> 173.09	-	Min. E.S.	19.00	1	Outflow Vol	,									
Saturation	96.2%	1				Gattion Vol	une per (cc)	1.00		USCS	СН					
Juturution	90.270	1			PERM	IEANT: D	eaired Tap	Water		0000	011	1				
TIME FUN	CTION	REAI	DINGS		TIMI	E IN MINUT	TES & SECO	ONDS			VOL	UME	PERMEABILITY			
DATE HOU	R MIN	Inflow	Outflow	Temp.	dt	dt	dt, acc	Head	(H1/H2)	Gradient	Inflow	Outflow	@ 20 Degrees C			
		(cc)	(cc)		(min)	(sec)	(sec)	(cm)	(inc.)		(cc)	(cc)	(cm/sec)			
03/15/17 13	26	0.0	25.0	20.6	0.0	0.0	0	235.44		29.98	0.00	0.00	0.0			
03/15/17 14	16	0.2	24.6	20.6	50.0	3000	3000	234.85	1.00	29.90	0.20	0.40	8.2E-08			
03/15/17 15	44	0.0	25.0	20.0	88.0	5280	8280	235.44	1.00	29.98	0.00	0.00	-			
03/16/17 7	38	1.7	22.8	18.7	954.0	57240	65520	231.64	1.02	29.49	1.70	2.20	2.9E-08			
03/16/17 9	18	1.8	22.6	19.3	100.0	6000	71520	231.34	1.00	29.46	0.10	0.20	2.1E-08			
03/16/17 10	8	1.9	22.5	19.3	50.0	3000	74520	231.15	1.00	29.43	0.10	0.10	2.9E-08 *			
03/16/17 11	52	2.1	22.3	19.3	104.0	6240	80760	230.76	1.00	29.38	0.20	0.20	2.7E-08 *			
03/16/17 14	19	2.4	22.1	19.3	147.0	8820	89580	230.27	1.00	29.32 29.30	0.30	0.20 0.10	2.4E-08 * 3.0E-08 *			
03/16/17 15	6	2.5	22.0	19.3	47.0	2820	92400	230.08	1.00	27.30	0.10	0.10	5.02-00			
Inflow	Pate	0.000030								*DI	RMFARI	ITV REPO	RTED AS 2.8E-08 cm/sec			
	w Rate	0.000030	1							TE	A TEADIL	ALL KETU				
	w Rate w/Inflow Ratio		]										DATE 3/15/17 CHECK DA REVIEW			
													APPROVE			

### GOLDER ASSOCIATES INC.



								LE WALL P ASTM D	5084							
						N	1ETHOD D	), CONSTAN	T RATE C	OF FLOW						
PROJECT TITL	LE	FTN/ENTI	ERGY INDEP	PENDENCE	/AR	1	Board #	8	C	OMMENTS						
PROJECT NUM	JECT NUMBER 1776956						Flow Pump	2								
SAMPLE ID						Flow P	ump Speed	11								
SAMPLE TYPE	MPLE TYPE UD					Technician SDM/PWM										
Sample Data, Ini	itial					Sample Dat	a Final			·						
Height, inches	Ittai					Height, incl	-	3.075					Sample		Sample	
Diameter, inches		2.866	Cell Pres.	94.0		Diameter, in		2.875		WATER C	ONTENTS		Initial		Final	
Area, cm <sup>2</sup>		41.62	Bot. Pres.	80.0		Area, cm <sup>2</sup>		41.88		Wt Soil & T		g	584.47		602.73	
Volume, cm <sup>3</sup>				Volume, cm	1 <sup>3</sup>	327.12		Wt Soil & T		g	425.80		434.04			
Mass, g			Mass, g		594.52		Wt Tare	-	g	0.00		8.33				
-	sture Content, % 37.27 Head, max. 182.18		Moisture C	ontent, %	39.63		Wt Moistur	re Lost	g	158.67		168.69				
Dry Density, pcf				Dry Density	y, pcf	81.22	Wt Dry Soil g 425.80					425.71				
pec. Gravity (assumed) 2.700 Max. Grad. 23.33			Volume Sol	ids, cm'	157.70	Water Content % 37.27%						39.63%				
olume Solids, c	2m <sup>3</sup>	157.70	Min. Grad.	23.33		Volume Voids, cm <sup>3</sup> 169.42										
Volume Voids, c	-	165.37	] '			Void Ratio		1.07								
Void Ratio		1.05	]			Saturation,	%	99.6%		DESCRIPT			_			
Saturation, %		96.0%	]							CLAY, trace fine to coarse sand; olive and brown.						
		Flow Pum	p Rate	1.18E-05	cm <sup>3</sup> /sec		USCS	СН								
					-											
Г	-		TIM	E FUNCTIO	ONS, SECO	NDS			dP							
	DATE	DAY	HOUR	MIN	ТЕМР	dt	dt,acc	dt	dt,acc	Reading	Head	Gradient	Permeability			
					(°C)	(min)	(min)_	(sec)	(sec)	(psi)	(cm)		(cm/sec)			
	03/22/17	42816	7	30	18.6	0	0	0	0	2.59	182.18	23.33	1.2E-08			
	03/22/17	42816	7	35	18.6	5	5	300	300	2.59	182.18	23.33	1.2E-08			
	03/22/17	42816	7	40	18.6	5	10	300	600	2.59	182.18	23.33	1.2E-08			
(	03/22/17	42816	7	45	18.6	5	15	300	900	2.59	182.18	23.33	1.2E-08	*		
	03/22/17	42816	7	50	18.6	5	20	300	1200	2.59	182.18	23.33	1.2E-08	*		
(	03/22/17	42816	7	55	18.6	5	25	300	1500	2.59	182.18	23.33	1.2E-08	*		
	03/22/17	42816	8	0	18.6	5	30	300	1800	2.59	182.18	23.33	1.2E-08	*		
. –	FRANSC	RIBED FRO	OM ORIGINA	AL DATA S	HEETS					PEI	RMEABILI	TY REPOR	TED AS ** 1.2E-08 c	n/sec **		
*]															DATE 3/	
*1															CHECK	
*']															REVIEW	

